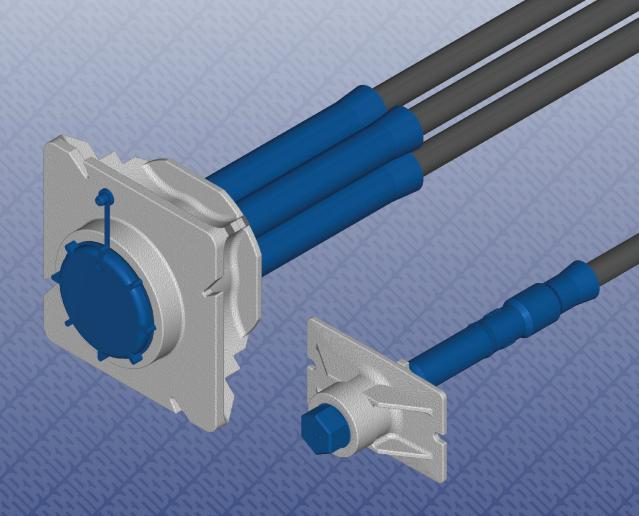


BR VT CONA CMM onded and Bonded Post-tensioning System

European Technical Assessment ETA – 06/0165





ETA-06/0165 BBR VT CONA CMM

Unbonded Post-tensioning System with 01, 02 and 04 Strands

BBR VT International Ltd

Ringstrasse 2, 8603 Schwerzenbach (Switzerland) www.bbrnetwork.com

0432-CPR-00299-1.2/1

11

Responsible BBR PT Specialist Company



The delivery note accompanying components of the BBR VT CONA CMM Post-tensioning System will contain the CE marking.



Assembly and installation of BBR VT CONA CMM tendons must only be carried out by qualified BBR PT Specialist Companies. Find the local BBR PT Specialist Company by visiting the BBR Network website www.bbrnetwork.com.



European Organisation for Technical Approvals Europäische Organisation für Technische Zulassungen Organisation Européenne pour l'Agrément technique

ETAG 013

Guideline for European Technical Approval of Post-tensioning Kits for Prestressing of Structures

CWA 14646

Requirements for the installation of post-tensioning kits for prestressing of structures and qualification of the specialist company and its personnel



BBR E-Trace is the trading and quality assurance platform of the BBR Network linking the Holder of Approval, BBR VT International Ltd, BBR PT Specialist Companies and the BBR Manufacturing Plant. Along with the established BBR Factory Production Control, BBR E-Trace provides effective supply chain management including installation, delivery notes and highest quality standards, as well as full traceability of components.









European **Technical Assessment**

ETA-06/0165 of 08.06.2018

General part

Technical Assessment Body issuing the European Technical Assessment

Österreichisches Institut für Bautechnik (OIB) Austrian Institute of Construction Engineering

Trade name of the construction product

BBR VT CONA CMM - Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands

Product family to which the construction product belongs

Bonded and unbonded post-tensioning kits for prestressing of structures with strands and monostrands

Manufacturer

BBR VT International Ltd Ringstrasse 2 8603 Schwerzenbach (ZH) Switzerland

Manufacturing plant

BBR VT International Ltd Ringstrasse 2 8603 Schwerzenbach (ZH) Switzerland

This European Technical Assessment contains

56 pages including Annexes 1 to 27, which form an integral part of this assessment.

This European Technical Assessment is issued in accordance with Regulation (EU) № 305/2011, on the basis of

EAD 160004-00-0301, European Assessment Document for Post-Tensioning Kits for Prestressing of Structures.

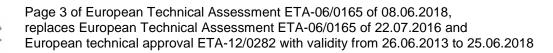
This European Technical Assessment replaces

European Technical Assessment ETA-06/0165 of 22.07.2016 and European technical approval ETA-12/0282 with validity from 26.06.2013 to 25.06.2018.



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Remarks

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Specific parts

1 Technical description of the product

1.1 General

The European Technical Assessment¹ – ETA – applies to a kit, the PT system

BBR VT CONA CMM – Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands,

comprising the following components, see Annex 1, Annex 2, Annex 3, Annex 4, Annex 5, and Annex 6.

Tendon

Unbonded tendons with 01, 02, or 04 tensile elements

Bonded tendons with 01 tensile element

Tensile element

Unbonded 7-wire prestressing steel strand with nominal diameters and maximum characteristic tensile strengths as given in Table 1, factory-provided with a corrosion protection system comprising corrosion protective filling material and HDPE-sheathing. Unbonded 7-wire prestressing steel strands are monostrands or VT CMM Bands.

7-wire prestressing steel strand with nominal diameters and maximum characteristic tensile strengths as given in Table 1 – Bare 7-wire prestressing steel strand are for unbonded and bonded tendons in ducts.

Table 1 Tensile elements

Nominal diameter	Nominal cross-sectional area	Maximum characteristic tensile strength 1)
mm	mm²	MPa
15.3	140	1 860
15.7	150	1 000
15.2 ²⁾	165	1 820

Prestressing steel strands with a characteristic tensile strength below 1 860 MPa may also be used.

NOTE $1 \text{ MPa} = 1 \text{ N/mm}^2$

²⁾ Compacted strand

ETA-06/0165 was firstly issued in 2006 as European technical approval with validity from 15.11.2006, extended in 2011 with validity from 15.11.2011, converted in 2016 to European Technical Assessment ETA-06/0165 of 22.07.2016, and amended in 2018 to European Technical Assessment ETA-06/0165 of 08.06.2018. ETA-12/0282 was firstly issued in 2013 as European technical approval with validity from 26.06.2006 and included in 2018 in European Technical Assessment ETA-06/0165 of 08.06.2018.

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Anchorage and coupler

Anchorage of the prestressing steel strands with ring wedges

End anchorage

Fixed (passive) anchor or stressing (active) anchor as end anchorage for 01, 02, and 04 prestressing steel strands

Stressing intermediate anchor for 01 prestressing steel strand

Fixed or stressing coupler

Sleeve coupler for 01 and 04 prestressing steel strands

- Helix and additional reinforcement in the region of the anchorage
- Corrosion protection for tensile elements, anchorages, and couplers

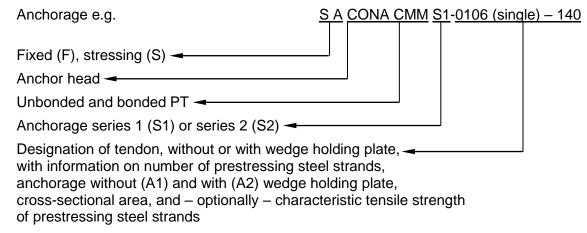
PT system

1.2 Designation and range of anchorages and couplers

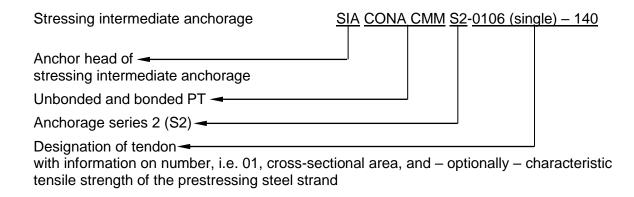
1.2.1 General

End anchorages can be fixed or stressing, or intermediate stressing anchorages. Couplers are fixed couplers only. The principal dimensions of anchorages and couplers are given in Annex 4, Annex 5, and Annex 6, and Annex 14 and Annex 15.

1.2.2 Designation

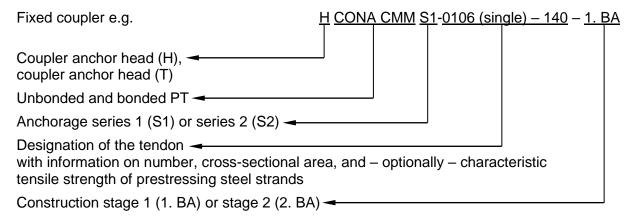


NOTE Wedge holding plates are available for S1-0206 (two) A2 and S1-0406 (four) A2.



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1.2.3 Anchorage

1.2.3.1 End anchorage

The anchor heads of the fixed and stressing anchorages are identical, see Annex 1, Annex 2, and Annex 3. A differentiation is needed for the construction works. The principal dimensions of the anchorages are given in Annex 4, Annex 5, and Annex 6.

Fixed anchorages that are accessible may be prelocked. Fixed anchorages that are not accessible are prelocked with prelocking force as specified in Table 5. The ring wedges are secured with rings between ring wedges and protection caps and wedge holding plates.

1.2.3.2 Stressing intermediate anchorage

The stressing intermediate anchorage is employed when structures are built in two or several steps. The principal dimensions of the stressing intermediate anchorage are given Annex 6.

The intermediate anchorage serves to stress and temporarily anchor one single prestressing steel strand in construction stage 1 – or construction stage n –, before construction stage 2 – or construction stage n + 1 - is built and the full length of the prestressing steel strand is stressed from the end of construction stage 2 - or construction stage n + 1. After stressing the prestressing steel strand from construction stage 2 – or construction stage n + 1 –, the stressing intermediate anchorage of construction stage 1 - or construction stage n - remains in the structure without taking any forces. Stressing intermediate anchorage is available for internal unbonded tendons with monostrands.

1.2.3.3 Restressable and exchangeable tendon

For tendons remaining restressable or exchangeable throughout the working life of the structure, grease, wax, or an equivalent soft filling material is used. This is applicable to

- Bare prestressing steel strand in a duct and
- Monostrands and VT CMM Bands.

A bonded tendon with bare prestressing steel strand, grouted in a duct is neither restressable nor exchangeable.

Significant to a restressable or exchangeable tendon is the excess length of the prestressing steel strands, see Annex 3. The extent of the excess length depends on the jack used for restressing and where applicable, the elongation for a full release of the prestressing force of the tendon. The protrusions of the prestressing steel strands require a permanent corrosive protection and an adapted protection cap.

Fixed and stressing coupler 1.2.4

Couplers are intended as fixed couplers only. The principal dimensions of the couplers are given in Annex 5, Annex 6, Annex 14, and Annex 15. Fixed couplers are for tendons with 01 and 04 prestressing steel strands, see Annex 1, Annex 2, and Annex 3.

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The prestressing force at the second construction stage may not be greater than that at the first construction stage, neither during construction, nor in the final state, nor due to any load combination.

For tendons CONA CMM S1, the tendon of construction stage 2 is coupled by screwing the coupler sleeve entirely on the threaded part of the coupler anchor head 1. BA (construction stage 1). The coupler anchor head 2. BA (construction stage 2) are prelocked with a prelocking force as specified in Table 5 and the wedges are secured by the following means.

- At coupler anchor head H CONA CMM S1-0106 (single) 2. BA (construction stage 2), with protection cap and wedge holding ring, see Annex 1
- At coupler anchor head H CONA CMM S1 -0406 (four) 2. BA (construction stage 2), with a wedge holding plate, see Annex 1.

For tendons CONA CMM S2, the coupler anchor head 2. BA (construction stage 2) is screwed onto the coupler anchor head 1. BA (construction stage 1). The tendon of construction stage 2 is coupled by pushing the strand in the installed coupler anchor head 2. BA (construction stage 2). Coupler plug and spring force the ring wedge and establish the connection. The wedge is secured by the following means.

 At coupler anchor head T CONA CMM S2-0106 (single) – 2. BA (construction stage 2), with spring and coupler plug, see Annex 2

1.2.5 Layout of the anchorage recesses

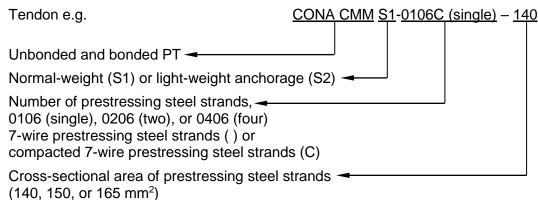
All anchor heads are placed perpendicular to the axis of the tendon, see Annex 14 and Annex 15.

In Annex 14, Annex 15, Annex 17, and Annex 18 the minimum dimensions of the anchorage recesses are given. The dimensions of the anchorage recesses are adapted to the prestressing jacks used. The ETA holder saves for reference information on the minimum dimensions of the anchorage recesses.

The formwork for the anchorage recesses should be slightly conical for ease of removal. The anchorage recesses are designed in such a way as to permit a reinforced concrete cover with the required dimensions, and in any case with a thickness of at least 20 mm.

Designation and range of the tendons 1.3

1.3.1 Designation



The characteristic tensile strength of the prestressing steel strands (1770, 1820, or 1860 MPa) may be indicated optionally.

Wedge holding plates are available for S1-0206 (two) A2 and S1-0406 (four) A2.

The available tendons with their respective anchorages and couplers are given in Table 2 and Table 3.

Table 2	Available tendons	 Anchorages 	and couplers
---------	-------------------	--------------------------------	--------------

Anaharaga and aqualar	Unbonde	Bonded tendon	
Anchorage and coupler	Monostrand 1)	Filled duct 2)	Grouted duct 2)
Stressing anchorage	+	+	+
Fixed anchorage – accessible	+	+	+
Fixed anchorage – not accessible	+	+	_
Intermediate anchorage	+ 3)	_	_
Fixed or stressing coupler	+ 4)	+	_

¹⁾ Tendon with 01, 02, and 04 monostrands

Key

+..... Tendon available

--......... Tendon unavailable

Table 3 Available tendons – Prestressing steel strands

Anchorage and coupler	Cross-sectional area of prestressing steel strand			
Anchorage and coupler	140 mm ²	150 mm ²	165 mm ²	
Stressing and fixed anchorage S1	+	+	+	
Stressing and fixed anchorage S2	+	+	_	
Intermediate anchorage S2		+		
Fixed or stressing coupler S1	+	+	+	
Fixed or stressing coupler S2	+	+	_	

Key

+..... Tendon available

--........ Tendon unavailable

1.3.2 Range

1.3.2.1 General

Prestressing and overstressing forces are given in the corresponding standards and regulations in force at the place of use. The maximum prestressing and overstressing forces are listed in Annex 13.

1.3.2.2 Unbonded tendon

The unbonded tendon comprises 01, 02, or 04 7-wire prestressing steel strands as monostrands or VT CMM Bands.

One single prestressing steel strand within a duct, filled with grease, wax, or an equivalent soft filling material, is also available as unbonded tendon.

²⁾ Tendon with 01 prestressing steel strand in duct

³⁾ Tendon with 01 monostrand

⁴⁾ Tendon with 01 and 04 monostrands

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1.3.2.3 Bonded tendon

The bonded tendon comprises one singe 7-wire prestressing steel strand, grouted within a duct, either in plastic or in steel.

1.3.2.4 CONA CMM S1-n06 - 140 and CONA CMM S2-0106 - 140

7-wire prestressing steel strand

Nominal diameter 15.3 mm

Nominal cross-sectional area 140 mm²

Maximum characteristic tensile strength...... 1 770 or 1 860 MPa

Annex 12 lists the available tendon range for CONA CMM S1-n06 – 140 and CONA CMM S2-0106 – 140.

1.3.2.5 CONA CMM S1-n06 - 150 and CONA CMM S2-0106 - 150

7-wire prestressing steel strand

Nominal diameter 15.7 mm

Nominal cross-sectional area 150 mm²

Maximum characteristic tensile strength...... 1 770 or 1 860 MPa

Annex 12 lists the available tendon range for CONA CMM S1-n06 – 150 and CONA CMM S2-0106 – 150.

1.3.2.6 CONA CMM S1-n06C - 165

Compacted 7-wire prestressing steel strand

Nominal diameter 15.2 mm

Nominal cross-sectional area 165 mm²

Maximum characteristic tensile strength......1 820 MPa

Annex 12 lists the available tendon range for CONA CMM S1-n06C - 165.

1.4 Duct

1.4.1 Use of ducts

Ducts are used for tendons with one single 7-wire prestressing steel strands in either unbonded or bonded applications.

1.4.1.1 Internal unbonded tendon with one single 7-wire prestressing steel strand

For unbonded tendons smooth steel or plastic ducts are used.

1.4.1.2 Internal bonded tendon with one single 7-wire prestressing steel strand

For bonded tendons in general corrugated ducts either in steel or in plastic are used. Smooth steel ducts may be used if permitted at the place of use.

1.4.2 Round steel strip sheath

Steel strip sheaths in conformity with EN 523^2 are used. The minimum radii of curvature, R_{min} , is according to Clause 1.9.

1.4.3 Pre-bent smooth round steel duct

If permitted at the place of use, smooth steel ducts according to EN 10255, EN 10216-1, EN 10217-1, EN 10219-1 or EN 10305-5 are used. The ducts are pre-bent and free of any kinks. The minimum radii of curvature, R_{min} , is according to Clause 1.9. The minimum wall thickness is according to the standards and regulations in force at the place of use.

² Reference documents are listed in Annex 26 and Annex 27.

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1.4.4 Corrugated plastic duct

Corrugated plastic ducts and minimum radii of curvature, R_{min} , according to the standards and regulations in force at the place of use are used.

1.4.5 Smooth plastic duct

Smooth plastic ducts according to EN 12201-1 and minimum radii of curvature, R_{min} , according to the standards and regulations in force at the place of use are used.

1.5 Friction losses

For calculation of loss of prestressing force due to friction Coulomb's law applies. Calculation of friction loss is by the equation

$$F_x = F_0 \cdot e^{-\mu \cdot (\alpha + k \cdot x)}$$

Where

F_x......kN.....Prestressing force at a distance x along the tendon

 F_0kN......Prestressing force at x = 0 m

μ...... rad-1.....Friction coefficient, see Table 4

 αrad.....Sum of angular displacements over distance x, irrespective of direction or sign

k rad/m......Wobble coefficient, see Table 4

NOTE 1 rad = 1 m/m = 1

Table 4 Friction parameters

	Recommer	Recommended values		Range of values	
Duct	μ	k	μ	k	
	rad ⁻¹	rad/m	rad ⁻¹	rad/m	
Steel strip sheath	0.18		0.17–0.19		
Smooth steel duct	0.18	0.005	0.16-0.24	0.004–0.007	
Corrugated plastic duct	0.12	0.005	0.10-0.14	0.004-0.007	
Smooth plastic duct	0.12		0.10-0.14		
Monostrand or VT CMM Band 140/150	0.06	8.73 · 10 ⁻³	0.05-0.07	0.007–0.011	
Monostrand or VT CMM Band 165	0.05	0.73 · 10 ·	0.04-0.06	0.007-0.011	

If band-shaped tendons CONA CMM S1-n06 - 150 or CONA CMM S1-n06 - 165 with two or four prestressing steel strands are installed upright, with flat-wise curvature and connected at support distances of 1.15 to 1.30 m, the wobble coefficient is $k = 4.37 \cdot 10^{-3}$ rad/m (= 0.25 °/m).

Friction losses in anchorages are low and do not have to be taken into consideration in design and execution.

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1.6 Support of tendons

The individual monostrands or VT CMM Bands are fastened in their position. Spacing of supports is.

1 Normally

Individual monostrands (01 strand) and

2 Free tendon layout in \leq 45 cm thick slabs

In the transition region between

- a) high tendon position and anchorage (e.g. cantilever)...... 1.50 m
- b) low and high tendon position or low tendon position and anchorage................. 3.00 m

In regions of high or low tendon position the tendons are connected in an appropriate way to the reinforcement mesh, at least at two points with a spacing of 0.3 to 1.3 m. The reinforcement mesh is fastened in its position. Special spacers for tendons are therefore not required. For details see Annex 19.

1.7 Slip at anchorages and couplers

Table 5 specifies the slip at anchorages couplers that is taken into consideration in calculations of tendon elongation and forces in tendon.

Table 5 Slip at anchorages and couplers

	SA CONA CMM S1-n06	6 mm		
	H CONA CMM S1-n06 – 1. BA	6 mm		
Stressing anchorage Coupler S1 and S2 1. BA	SA CONA CMM S2-0106 (single)			
Couplet of and 52 1. BA	T CONA CMM S2-0106 (single) – 1. BA	8 mm		
	SIA CONA CMM S2-0106 (single)			
Not accessible fixed anchorage,	FA CONA CMM S1-n06	2		
prelocked 1)	H CONA CMM S1-n06 – 2. BA	3 mm		
Coupler S1 2. BA, prelocked 1)	FA CONA CMM S2-0106 (single)	4 mm		
	FA CONA CMM S1-0106 (single)	6 mm		
	FA CONA CMM S2-0106 (single)			
Accessible passive anchorage Coupler S2 2. BA	T CONA CMM S2-0106 (single) – 2. BA	8 mm		
Couplet 32 2. BA	FA CONA CMM S1-0206 (two)	2)		
	FA CONA CMM S1-0406 (four)	8 mm ²⁾		

¹⁾ Prelocked with ~ 0.5 · Fpk

Y1860S7 9 mm Y1820S7G 7 mm

1.8 Centre spacing and edge distances for anchorages

In general, spacing and distances are not less than the values given in Table 6 and Annex 14 and Annex 15.

However, a reduction of up to 15 % of the centre spacing of tendon anchorages in one direction is permitted, but should not be less than the outside diameter of the helix and placing of additional

²⁾ If a more exact evaluation is required, slip for

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reinforcement still is possible, see Annex 16. In this case the spacing in the perpendicular direction is increased by the same percentage. The corresponding edge distance is calculated by

$$a_e = \frac{a_c}{2} - 10 \text{ mm} + c$$

$$a_{\underline{e}} = \frac{a_{\underline{c}}}{2} - 10 \text{ mm} + c$$

$$b_e = \frac{b_c}{2} - 10 \text{ mm} + c$$
 $b_e = \frac{b_c}{2} - 10 \text{ mm} + c$

$$b_{\underline{e}} = \frac{b_{\underline{c}}}{2} - 10 \text{ mm} + c$$

Where

ac, ac......mm......... Centre spacing before and after modification

bc, bc.........mm............ Centre spacing in the direction perpendicular to ac before and after modification

ae, ag........mm....... Edge distance before and after modification

modification

cmm.......... Concrete cover

Standards and regulations on concrete cover in force at the place of use are observed.

The minimum values for a_c, b_c, a_e, and b_e are given in Table 6 and in Annex 14 and Annex 15.

Table 6 Spacing of tendon anchorages

Tendon			CONA CMM S1-0106	CONA CMM S2-0106 1)	CONA CMM S2-0106 ²⁾	CONA CMM S1-0206	CONA CMM S1-0406
Minimum centre spacing	a _c , b _c	mm	180, 140	175, 105	235, 150	200, 150	300, 220
Minimum edge distance	a _e , b _e	mm	70 + c, 50 + c	78 + c, 43 + c	108 + c, 65 + c	90 + c, 65 + c	130 + c, 90 + c

With helix or additional reinforcement

c...... Concrete cover

Standards and regulations on concrete cover in force at the place of use are observed.

1.9 Minimum radii of curvature of internal tendons

Minimum radii of curvature for tendons with 7-wire prestressing steel strands

The minimum radius of curvature, R_{min}, of internal tendons in steel strip sheath is

$$R_{min} = \begin{cases} \geq \frac{F_{pm, 0}}{p_R} \\ \text{and} \\ \geq \frac{400 \cdot d}{3000} \end{cases}$$

Where

R_{min} Minimum radii of curvature

 $F_{pm, 0}$kN....... Prestressing force of the tendon

d......mm....... Nominal diameter of the prestressing steel strand

p_R......kN/m...... Design pressure under the prestressing steel strands

Without helix and without additional reinforcement. Concrete cover refers to reinforcement, placed in the same cross section, however 10 mm as a minimum.

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The above given equations on the minimum radius of curvature result in a minimum radius of 2.0 m applying the following parameters.

- a prestressing force of the tendon of F_{pm, 0} = 0.85 ⋅ F_{p0.1}
- a nominal diameter of the prestressing steel strand of d = 15.3 mm, d = 15.7 mm, and d = 15.2 mm
- a pressure under the prestressing strands of $p_R = 140 \text{ kN/m}$ or $p_R = 200 \text{ kN/m}$ and
- a concrete compressive strength of f_{cm, 0, cube} ≥ 18 MPa.

In case of different tendon parameters or a different pressure under the prestressing strands, p_R , the calculation of the minimum radius of curvature of the tendon is carried out using the equation given above.

Depending on the concrete strength at the time of stressing, additional reinforcement for splitting forces may be required in the areas of reduced minimum radii of curvature.

Standards and regulations on minimum radii of curvature or on the pressure under the prestressing strands in force at the place of use are observed.

1.9.2 Minimum radii of curvature for tendons with monostrands or VT CMM Bands

The minimum radius of curvature R_{min} of internal tendons with prestressing steel strands of nominal diameter of 15.7 or 15.2 mm is 2.5 m. If this radius is adhered to, verification of prestressing steel outer fibre stresses in curved sections is not required. The minimum radius of curvature for deviation of a tendon with multistrand anchorages in the anchorage zone outside the transition pipes up to the last reinforcement steel bar is 3.5 m.

1.10 Concrete strength at time of stressing

Concrete in conformity with EN 206 is used. At the time of stressing the mean concrete compressive strength, $f_{\text{cm},\,0}$, is at least

S1 anchorages

 $f_{cm. 0. cube 150} \ge 24 MPa or$

 $f_{\text{cm, 0, cylinder}} \geq 20 \text{ MPa}$

S2 anchorages with helix or additional reinforcement

 $f_{\text{cm, 0, cube 150}} \ge 22 \text{ MPa or}$

 $f_{cm, 0, cylinder} \ge 18 MPa$

S2 anchorages with without additional reinforcement

 $f_{\text{cm, 0, cube 150}} \ge 23 \text{ MPa or}$

 $f_{\text{cm, 0, cylinder}} \geq 19 \text{ MPa}$

The concrete test specimens are subjected to the same curing conditions as the structure.

For partial prestressing with 30 % of the full prestressing force the actual mean value of the concrete compressive strength is at least $0.5 \cdot f_{\text{cm}, 0, \text{cube}}$ or $0.5 \cdot f_{\text{cm}, 0, \text{cylinder}}$. Intermediate values may be interpolated linearly according to Eurocode 2.

Helix, additional reinforcement, centre spacing and edge distance are taken from Annex 14 and Annex 15, see also the Clauses 1.12.5 and 2.2.3.4.

Where

 $f_{\text{cm, 0, cube 150}}$Mean concrete compressive strength at time of stressing, determined at cubes, 150 mm

 $f_{\text{cm, 0, cylinder} \, \varnothing \, 150}$Mean concrete compressive strength at time of stressing, determined at cylinders, diameter 150 mm

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Components

1.11 Prestressing steel strands

Only 7-wire prestressing steel strands with characteristics according to Table 7 are used, see also Annex 11.

Table 7 Prestressing steel strands

Maximum characteristic tensile strength 1)	f_{pk}	MPa	18	60	1 820
Nominal diameter	d	mm	15.3	15.7	15.2 ²⁾
Nominal cross-sectional area	A_p	mm ²	140	150	165
Mass of prestressing steel	М	kg/m	1.093	1.172	1.289
Sheathed prestressing steel strand, filled with a corrosion protective filling material – Individual monostrands or VT CMM Bands					
Nominal mass per prestressing steel strand		kg/m	1.23	1.31	1.42
External diameter of HDPE-sheathing		mm	≥ 19.5	≥ 20	≥ 19.5

¹⁾ Prestressing steel strands with a characteristic tensile strength below 1 860 MPa may also be used.

Sheathed prestressing steel strands provided with a corrosion protective filling material may be either individual monostrands or VT CMM Bands.

In the course of preparing the European Technical Assessment, no characteristic has been assessed for prestressing steel strands. In execution, a suitable prestressing steel strand that conforms to Annex 11 and is according to the standards and regulations in force at the place of use is taken.

1.12 Anchorages and couplers

1.12.1 General

The components of anchorages and couplers are in conformity with the specifications given in Annex 4, Annex 5, Annex 6, Annex 7, Annex 8, and Annex 9 and the technical file³. Therein the component dimensions, materials and material identification data with tolerances are given.

1.12.2 Anchor head

1.12.2.1 General

The anchor heads, see Annex 4 and Annex 6, are made of cast iron with spheroidal graphite. They provide regularly arranged conical holes to accommodate 01, 02, or 04 prestressing steel strands and ring wedges. The load transfer to the concrete occurs in two planes for anchorages S1 and in one plane for anchorage S2. Threaded blind bores may be provided at anchor heads A CONA CMM S1-0206 (two) A2 and A CONA CMM S1-0406 (four) A2 to attach wedge retaining plates.

The anchor heads have cylindrical extensions with an internal thread to screw-in a protection cap. The outlet end of the conical holes for anchoring the prestressing steel strands is formed in such a way as to allow the transition pipes to be inserted tension-proof.

1.12.2.2 Anchor head in unbonded tendon

For unbonded tendon a protection cap is screwed in the cylindrical extension of the anchor head, filled with corrosion protective filling material to protect ring wedges and prestressing steel strands.

²⁾ Compacted strand

The technical file of the European Technical Assessment is deposited at Österreichisches Institut für Bautechnik.

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Transition pipes fitted at the outlet end of the conical holes act as the transition from the anchor head to the sheathing of the prestressing steel strands for monostrand tendons, see Annex 1, Annex 2, Annex 7, and Annex 8. For unbonded tendons in ducts, transition pipes fitted at the outlet end of the conical holes act as the transition from the anchor head to the duct of the tendon, see Annex 3 and Annex 9. The transition pipes are provided with a port to attach a hose for filling or venting.

1.12.2.3 Anchor head in bonded tendon

During grouting of the bonded tendon, a protection cap is screwed in the cylindrical extension of the anchor head filled with grout to protect ring wedges and prestressing steel strands.

Grouting transition pipes fitted at the outlet end of the conical holes act as the transition from the anchor head to the duct of the tendon, see Annex 3 and Annex 9. The grouting transition pipes are provided with a port to attach a hose for grouting or venting.

1.12.2.4 Anchor head for stressing intermediate anchorage

The anchor head SIA CONA CMM S2-0106 (single) is made of cast iron with spheroidal graphite, see Annex 6. It provides a conical hole to accommodate 01 prestressing steel strand and ring wedge I. The load transfer to the concrete occurs in one plane.

The anchor head has a cylindrical extension with an internal thread to screw-in a protection pipe. The outlet end of the conical holes for anchoring the prestressing steel strands is formed in such a way as to allow a transition pipe to be inserted tension-proof, see Annex 2 and Annex 8.

Stressing intermediate anchorage is for unbonded tendons. At the outlet end of the conical hole a transition pipe is fitted that acts as transition from anchor head to sheathing of the prestressing steel strands. The protection pipe is screwed in the cylindrical extension of the anchor head that, together with a plastic pipe, or cut sheathing and tape acts as transition to the sheathing of the strands, see Annex 2 and Annex 8.

1.12.3 Coupler

1.12.3.1 Coupler S1

Fixed couplers are provided for tendons with 01 and 04 prestressing steel strands. They comprise a coupler anchor head 1. BA (construction stage 1) and a coupler anchor head 2. BA (construction stage 2).

The coupler anchor head 1. BA (construction stage 1) is of the same shape as the anchor heads of active and passive anchorages for 01 and 04 prestressing steel strands. Compared to the anchor heads of active and passive anchorages, the cylindrical extension is deeper, provides an external thread to accommodate the coupler sleeve and an internal thread to screw-in a protection cap, see Annex 5.

The coupler anchor head 2. BA (construction stage 2) for 01 prestressing steel strand is either a cast iron head with a conical hole or a steel body with a conical bore. The coupler anchor head 2. BA (construction stage 2) for 04 prestressing steel strands is a steel body with conical bores. All coupler anchor heads provide an external thread for the coupler sleeve, see Annex 5.

The connection between coupler anchor heads 1. BA (construction stage 1) and 2. BA (construction stage 2) is by means of a coupler sleeve, a steel tube featuring an internal thread, a threaded bore to accommodate the filling device and a bore for ventilation, see Annex 1 and Annex 5.

Transition pipes fitted at the outlet end of the conical holes act as the transition from the coupler anchor head 1. BA (construction stage 1) and 2. BA (construction stage 2) to the sheathing of the prestressing steel strands, see Annex 1 and Annex 7.

The end surface of the fixed coupler H CONA CMM 0406 (four) is provided with a BDSD-plate to permit settlement of the coupler during stressing, see Annex 14 and Annex 9.

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1.12.3.2 Coupler S2

The fixed coupler comprises a coupler anchor head 1. BA (construction stage 1) and a coupler anchor head 2. BA (construction stage 2).

The coupler anchor head 1. BA (construction stage 1) is of the same shape as the anchor heads of active and passive anchorages. Compared to the anchor heads of active and passive anchorages, the cylindrical extension is deeper, provides an external thread to accommodate the coupler anchor head 2. BA (construction stage 2) and an internal thread to screw-in a protection cap, see Annex 2 and Annex 6.

The coupler anchor head 2. BA (construction stage 2) is a cast iron head with a conical hole for prestressing steel strand and wedge. The outlet end of the conical holes is formed in such a way as to allow a transition pipe to be inserted tension-proof. The opposite end of the coupler anchor head 2. BA (construction stage 2) is provided with an internal thread to be screwed onto the coupler anchor head 1. BA (construction stage 1). Thereby, coupler anchor head 1. BA (construction stage 2) are connected. The prestressing steel strand of the 2. BA (construction stage 2) is pushed in the installed coupler anchor head 2. BA (construction stage 2). Holding spring and coupler plug force the wedge and establish and secure the connection of both tendons, see Annex 2, Annex 3, and Annex 6.

For unbonded monostrand tendons, transition pipes fitted at the outlet end of the conical holes act as the transition from the coupler anchor head 1. BA (construction stage 1) and 2. BA (construction stage 2) to the sheathing of the prestressing steel strands, see Annex 2 and Annex 8.

For unbonded tendons in ducts, transition pipes fitted at the outlet ends of the conical holes act as the transition from the coupler anchor heads to the ducts of the tendons. The transition pipes are provided with a port to attach a hose for filling or venting, see Annex 3 and Annex 9.

1.12.4 Ring wedges

The ring wedges, see Annex 5 and Annex 6, are in either two pieces or three pieces. Three different ring wedges are used.

- Ring wedge H in three pieces, fitted with spring ring, is available in two different materials.
- Ring wedge F in three pieces, without spring ring or fitted with spring ring, is made of one material.
- Ring wedge I in two pieces, without spring ring or fitted with spring ring, is made of one material. Ring wedge I is for stressing intermediate anchorage only.

Within one anchorage or coupler, only one of these ring wedges is used.

Devices for securing the ring wedges after prelocking are.

- At inaccessible fixed anchorage FA CONA CMM S1-0106, FA CONA CMM S1-0206 (two) A1 and FA CONA CMM S1-0406 (four) A1 wedge holding rings with protection cap are installed.
- At inaccessible fixed anchorage FA CONA CMM S1-0206 (two) A2 and FA CONA CMM S1-0406 (four) A2 a wedge holding plate is attached.
- At coupler H CONA CMM S1-0106 (single) 2. BA wedge holding ring with protection cap are installed.
- At coupler H CONA CMM S1-0406 (four) 2. BA a wedge holding plate is attached.
- At coupler T CONA CMM S2-0106 (single) 2. BA coupler plug and spring are forcing the ring wedge.

1.12.5 Helix and additional reinforcement

Helix and the additional reinforcement are made of ribbed reinforcing steel. The end of the helix on the anchorage side is welded to the following turn. The helix is placed in the tendon axis.

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Dimensions of helix and additional reinforcement conforms to the values specified in Annex 14 and Annex 15, see also Clause 2.2.3.4.

If required for a specific project design, the reinforcement given in Annex 14 and Annex 15 may be modified in accordance with the respective regulations in force at the place of use as well as with the relevant approval of the local authorities and of the ETA holder, to provided equivalent performance.

1.12.6 Protection cap and coupler plug

Protection cap and coupler plug are made of plastic, see Annex 8 and Annex 9.

The protection cap is screwed into the anchor head and for anchorages with two or four prestressing steel strands is provided with an air vent, Annex 1 and Annex 2.

The coupler plug is screwed into coupler anchor head T CONA CMM S2-0106 (single) -2. BA together with a spring to force the wedge, see Annex 2.

1.12.7 Pocket former set

The pocket former set is made of plastic. It comprises mandrel, nut, and pocket former, see Annex 7 and Annex 8.

Available are

For anchorage S1 and S2 with 01 prestressing steel strand

Mandrel, suitable for anchorage S1 and S2

Nut, suitable for anchorage S1 and S2

One pocket former, suitable for anchorage S1 and S2

One pocket former for anchorage S1

One pocket former for anchorage S2

The three pocket former are of different dimensions.

For anchorage S1 with 02 and 04 prestressing steel strands

One common mandrel for 02 and 04 prestressing steel strands

One common nut for 02 and 04 prestressing steel strands

One common pocket former for 02 and 04 prestressing steel strands

The pocket formers are employed to form anchorage recesses for stressing anchorages SA CONA CMM S1-0106, SA CONA CMM S2-0106, SA CONA CMM S1-0206, and SA CONA CMM S1-0406.

1.12.8 Material specifications

Annex 10 lists the material standards or specifications of the components.

1.13 Permanent corrosion protection

1.13.1 General

In the course of preparing the European Technical Assessment no characteristic has been assessed for components and materials of the corrosion protection system referred to in the Clauses 1.13.2 and 1.13.3. In execution, all components or materials are selected according to the standards and regulations in force at the place of use.

1.13.2 Unbonded tendon

1.13.2.1 Corrosion protection of prestressing steel strand

Monostrands or VT CMM Bands are sheathed in the factory with an extruded HDPE-sheathing with a thickness of at least 1.0 mm. The actual thickness of the sheathing is in accordance with the standards and regulations in force at the place of use. Inside the HDPE-sheathing a corrosion protective filling material is applied. When assembling the anchorage, the sheathing is

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removed along the required length. During construction the prestressing steel strand excess lengths protruding from the anchorage are temporarily protected with cut-off HDPE-sheaths.

Beside monostrands, one single prestressing steel strand can be installed unbonded in a duct. The duct is filled with grease, wax, or an equivalent soft material.

1.13.2.2 Corrosion protection in anchorage and coupler

All voids of anchorages and couplers are completely filled with corrosion protective filling material according to the installation instructions in Annex 20.

During tendon installation not accessible anchorage, fixed or stressing coupler – and where appropriate accessible anchorage as well – are completely filled with corrosion protective filling material. I.e. the void from protection cap or coupler plug to wedge and up to the inlet or venting port of the transition pipe is provided with corrosion protective filling material.

Anchorages that are prelocked receive their corrosion protection immediately after the prelocking operation by filling with corrosion protective filling material and screwing-on of the protection cap. Coupler sleeve of fixed and stressing coupler S1 and coupler anchor head 2. BA of fixed and stressing coupler S2 is completely filled with corrosive protective filling material during installation.

All anchorages and couplers are fully embedded in concrete. The recesses are designed as to permit a reinforced concrete cover with the required dimensions and in any case with a thickness of at least 20 mm.

1.13.3 Bonded tendon

To protect the tendons from corrosion, ducts and anchorages are completely filled with grout according to EN 447, special grout according to EAD 160027-00-0301, or special grout as applicable at the place of use. This is ensured by gout penetrating from the protection caps at the anchorages.

All anchorages are fully embedded in concrete. The recesses are designed as to permit a reinforced concrete cover with the required dimensions and in any case with a thickness of at least 20 mm.

2 Specification of the intended uses in accordance with the applicable European Assessment Document (hereinafter EAD)

2.1 Intended uses

The PT system is intended to be used for the prestressing of structures. The specific intended uses are listed in Table 8.

Table 8 Intended uses

Line №	Use category		
Use categories according to tendon configuration and material of structure			
1	Internal bonded tendon for concrete and composite structures		
2	Internal unbonded tendon for concrete and composite structures		

2.2 Assumptions

2.2.1 General

Concerning product packaging, transport, storage, maintenance, replacement, and repair it is the responsibility of the manufacturer to undertake the appropriate measures and to advise his clients on transport, storage, maintenance, replacement, and repair of the product as he considers necessary. Page 21 of European Technical Assessment ETA-06/0165 of 08.06.2018, replaces European Technical Assessment ETA-06/0165 of 22.07.2016 and European technical approval ETA-12/0282 with validity from 26.06.2013 to 25.06.2018



2.2.2 Packaging, transport and storage

Advice on packaging, transport, and storage includes.

- During transport of prefabricated tendons, a minimum diameter of curvature of 1.45 to 1.75 m or as specified by the manufacturer of the prestressing steel strand is observed.
- Temporary protection of prestressing steel and components in order to prevent corrosion during transport from production site to job site
- Transportation, storage and handling of the prestressing steel and other components in a manner as to avoid damage by mechanical or chemical impact
- Protection of prestressing steel and other components from moisture
- Keeping tensile elements separate from areas where welding operations are performed

2.2.3 Design

2.2.3.1 General

It is the responsibility of the ETA holder to ensure that all necessary information on design and installation is submitted to those responsible for the design and execution of the structures executed with "BBR VT CONA CMM – Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands".

Design of the structure permits correct installation and stressing of the tendons. The reinforcement in the anchorage zone permits correct placing and compacting of concrete.

2.2.3.2 Anchorage Recess

The anchorage recess is designed so as to permit a concrete cover of at least 20 mm at the protection caps in the final state.

Clearance is required for the handling of prestressing jacks. In order to allow for imperfections and to ease the cutting of the prestressing steel strand excess lengths, it is recommended to increase the dimensions of the recesses. The forms for the recesses should be slightly conical for easy removal.

If other prestressing jacks than those shown in Annex 17 and Annex 18 are used, the ETA holder keeps information on prestressing jacks and minimum dimensions of anchorage recesses.

In case of failure, bursting out of prestressing steels is prevented. Sufficient protection is provided by e.g. a cover of reinforced concrete.

2.2.3.3 Maximum prestressing forces

Prestressing and overstressing forces are specified in the respective standards and regulations in force at place of use. Annex 13 lists the maximum possible prestressing and overstressing forces according to Eurocode 2.

2.2.3.4 Centre spacing, edge distance, and reinforcement in the anchorage zone

Centre spacing, edge distance, helix, and additional reinforcement given in Annex 14 and Annex 15 are adopted, see Clause 1.8.

Verification of transfer of prestressing forces to structural concrete is not required if centre spacing and edge distance of anchorages and couplers as well as grade and dimensions of additional reinforcement, see Annex 14 and Annex 15, are conformed to. In the case of grouped anchorages the additional reinforcement of the individual anchorages can be combined, provided appropriate anchorage is ensured. However, number, cross-sectional area, and position with respect to the anchor heads remain unchanged.

The reinforcement of the structure is not employed as additional reinforcement. Reinforcement exceeding the required reinforcement of the structure may be used as additional reinforcement, provided appropriate placing is possible.

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The forces outside the area of the additional reinforcement are verified and, if necessary, dealt with by appropriate reinforcement.

If required for a specific project design, the reinforcement given in Annex 14 and Annex 15 may be modified in accordance with the respective regulations in force at the place of use as well as with the relevant approval of the local authority and of the ETA holder to provide equivalent performance.

2.2.3.5 Tendons in masonry structures – load transfer to the structure

Post-tensioning kits are primarily used in structures made of concrete. They can, however, be used with other structural materials, e.g. in masonry structures. However, there is no particular assessment in EAD 160004-00-0301 for these applications. Hence, load transfer of stressing force from the anchorage to masonry structures is via concrete or steel members, designed according to the European Technical Assessment, especially according to the Clauses 1.8, 1.10, 1.12.5 and 2.2.3.4, or according to Eurocode 3, respectively.

The concrete or steel members have dimensions as to permit a force of $1.1 \cdot F_{pk}$ being transferred into the masonry. The verification is according to Eurocode 6 as well as to the respective standards and regulations in force at the place of use.

2.2.4 Installation

2.2.4.1 General

It is assumed that the product will be installed according to the manufacturer's instructions or – in absence of such instructions – according to the usual practice of the building professionals.

Assembly and installation of tendons is only carried out by qualified PT specialist companies with the required resources and experience in the use of multi strand unbonded post-tensioning systems, see CWA 14646. The respective standards and regulations in force at the place of use are considered. The company's PT site manager has a certificate, stating that she or he has been trained by the ETA holder and that she or he possesses the necessary qualifications and experience with the "BBR VT CONA CMM – Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands".

The sequence of work steps for installation of anchorage and fixed coupler is described in Annex 20 and the construction stages are shown in Annex 21 and Annex 22

The tendons may be manufactured on site or in the factory, i.e. prefabricated tendons. The tendons are carefully handled during production, transport, storage, and installation. The corrosion protected HDPE sheathed prestressing steel strands are usually delivered to site in coils with an internal diameter of 1.45 to 1.75 m.

Anchor heads, and coupler anchor heads are placed perpendicular to the tendon's axis. Couplers are situated in a straight tendon section.

In the anchorage zone, the webs of the VT CMM Bands are longitudinally cut over a length of 1.3 m from the end. The layout of the transition zone is shown in Annex 19.

Before placing the concrete a final check of the installed tendons is carried out. At that time, the passive anchorages mounted at the PT works are randomly checked for proper seating of the ring wedges and complete filling of the protection caps with corrosion protective filling material. In the case of minor damage of the sheathing, the damaged area is cleaned and sealed with an adhesive tape.

2.2.4.2 Stressing operation

With a mean concrete compressive strength in the anchorage zone according to the values laid down in Annex 14 and Annex 15 full prestressing may be applied.

Stressing and, if applicable, wedging is carried out using a suitable prestressing jack. The wedging force corresponds to approximately 25 kN per wedge.

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Elongation and prestressing forces are checked continuously during the stressing operation. The results of the stressing operation are recorded and the measured elongations compared with the prior calculated values.

After releasing the prestressing force from the prestressing jack, the tendon is pulled in and reduces the elongation by the amount of slip at the anchor head of the stressing anchorage.

Information on the prestressing equipment has been submitted to Österreichisches Institut für Bautechnik. The ETA holder keeps available information on prestressing jacks and appropriate clearance behind the anchorage.

The safety-at-work and health protection regulations shall be complied with.

2.2.4.3 Restressing

Restressing of tendons in combination with release and reuse of wedges is permitted, whereas the wedges bite into a least 15 mm of virgin strand surface and no wedge bite remains inside the final length of the tendon between anchorages.

Tendons with 7-wire prestressing steel strands that remain restressable throughout the working life of the structure are unbonded tendons. Grease, wax, or an equivalent soft material is used as filling material for tendons in ducts. Moreover, a strand protrusion at the stressing anchor remains with a length compatible with the prestressing jack used.

Restressing of tendons with monostrand or VT CMM Bands is possible.

2.2.4.4 Exchanging tendons

Exchange of unbonded tendons is permitted, subject of acceptance at the pace of use. The specifications for exchangeable tendons are defined during the design phase.

Corrosion protective filling material for tendons in ducts is grease, wax, or an equivalent soft material. Exchanging the prestressing steel strand of monostrand or VT CMM Band, with the sheathing remaining in the structure is also possible.

The radii of curvature should be reasonable larger than the minimum radii given in Clause 1.9 as to not impair the plastic ducts or sheathings of monostrand or VT CMM Band by wear due to stressing of the tendons.

Stressing and fixed anchorages are accessible and adequate space is provided behind the anchorages. Moreover, a strand protrusion at the stressing anchor remains with a length compatible with the prestressing jack used and allowing for a safe release of the complete prestressing force.

2.2.4.5 Filling operations

2.2.4.5.1 Grouting

Grout is injected through the inlet holes until it escapes from the outlet tubes with the same consistency. At fixed and stressing anchorages grout penetrates from the protection caps to ensure complete filling around the wedges. To avoid voids in the hardened grout special measures are applied for long tendons, tendon paths with distinct high points, or inclined tendons. All vents, grouting inlets, and protection caps are sealed immediately after grouting.

The standards observed for cement grouting in prestressing ducts are EN 445, EN 446, and EN 447 or the standards and regulations in force at the place of use are applied for ready mixed grout.

2.2.4.5.2 Filling with corrosion protective filling material

Unbonded tendons with ducts are with corrosion protective filling material. The specifications and the recommendations of the supplier are relevant for corrosion protective filling material.

Filling with corrosion protective filling material follows a similar procedure as the one specified for grouting. At anchorages and couplers, the voids from wedge to port are completely filled with corrosion protective filling material prior to filling operation, preferably during tendon installation.

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2.2.4.5.3 Filling records

The results of the grouting and filling operation are recorded.

2.2.4.6 Welding

Welding is not intended and it is not permitted to weld on built-in components of post-tensioning systems.

In case of welding operations near tendons precautionary measures are required to avoid damage to the corrosion protection system.

2.3 Assumed working life

The European Technical Assessment is based on an assumed working life of the BBR VT CONA CMM – Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands of 100 years, provided that the BBR VT CONA CMM – Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands is subject to appropriate installation, use, and maintenance, see Clause 2.2. These provisions are based upon the current state of the art and the available knowledge and experience.

In normal use conditions, the real working life may be considerably longer without major degradation affecting the basic requirements for construction works⁴.

The indications given as to the working life of the construction product cannot be interpreted as a guarantee, neither given by the product manufacturer or his representative nor by EOTA nor by the Technical Assessment Body, but are regarded only as a means for expressing the expected economically reasonable working life of the product.

3 Performance of the product and references to the methods used for its assessment

3.1 Essential characteristics

The performances of the PT system for the essential characteristics are given in Table 9. In Annex 25 the combinations of essential characteristics and corresponding intended uses are listed.

 Table 9
 Essential characteristics and performances of the product

Nº	Essential characteristic	Product performance			
Product BBR VT CONA CMM					
Intended use The PT system is intended to be used for the prestressing of structures, Clause 2.1, Table 8, lines № 1 and 2.					
Basic requirement for construction works 1: Mechanical resistance and stability					
1	Resistance to static load	See Clause 3.2.1.1.			
2	Resistance to fatigue	See Clause 3.2.1.2.			
3	Load transfer to the structure	See Clause 3.2.1.3.			
4	Friction coefficient	See Clause 3.2.1.4.			

⁴ The real working life of a product incorporated in a specific works depends on the environmental conditions to which that works are subject, as well as on the particular conditions of design, execution, use, and maintenance of that works. Therefore, it cannot be excluded that in certain cases the real working life of the product may also be shorter than the assumed working life.



Nº	Essential characteristic	Product performance		
5	Deviation, deflection (limits) for internal bonded and unbonded tendon	See Clause 3.2.1.5.		
6	Assessment of assembly	See Clause 3.2.1.6.		
7	Corrosion protection	See Clause 3.2.1.7.		
Basic requirement for construction works 2: Safety in case of fire				
8	Reaction to fire	See Clause 3.2.2.1.		
Basic requirement for construction works 3: Hygiene, health and the environment				
9	Content, emission and/or release of dangerous substances	See Clause 3.2.3.1.		
Basic requirement for construction works 4: Safety and accessibility in use				
_	Not relevant. No characteristic assessed.			
Basic requirement for construction works 5: Protection against noise				
_	Not relevant. No characteristic assessed.	_		
Basic requirement for construction works 6: Energy economy and heat retention				
_	Not relevant. No characteristic assessed.	_		
Basic requirement for construction works 7: Sustainable use of natural resources				
	No characteristic assessed.	_		

3.2 Product performance

- 3.2.1 Mechanical resistance and stability
- 3.2.1.1 Resistance to static load

The PT system as described in the ETA meets the acceptance criteria of EAD 160004-00-0301, Clause 2.2.1. The characteristic values of maximum force, F_{pk} , of the tendon for prestressing steel strands according to Annex 11 are listed in Annex 12.

3.2.1.2 Resistance to fatigue

The PT system as described in the ETA meets the acceptance criteria of EAD 160004-00-0301, Clause 2.2.2. The characteristic values of maximum force, F_{pk} , of the tendon for prestressing steel strands according to Annex 11 are listed in Annex 12.

3.2.1.3 Load transfer to the structure

The PT system as described in the ETA meets the acceptance criteria of EAD 160004-00-0301, Clause 2.2.3. The characteristic values of maximum force, F_{pk} , of the tendon for prestressing steel strands according Annex 11 are listed in Annex 12.

3.2.1.4 Friction coefficient

For friction losses including friction coefficient see Clause 1.5.

3.2.1.5 Deviation, deflection (limits) for internal bonded and unbonded tendon

For minimum radii of curvature see Clause 1.9.

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3.2.1.6 Assessment of assembly

The PT system as described in the ETA meets the acceptance criteria of EAD 160004-00-0301, Clause 2.2.7.

3.2.1.7 Corrosion protection

The PT system as described in the ETA meets the acceptance criteria of EAD 160004-00-0301, Clause 2.2.13.

3.2.2 Safety in case of fire

3.2.2.1 Reaction to fire

The performance of components made of steel or cast iron is Class A1 without testing.

The performance of components of other materials has not been assessed.

3.2.3 Hygiene, health and the environment

3.2.3.1 Content, emission and/or release of dangerous substances

According to the manufacturer's declaration, the PT system does not contain dangerous substances.

SVOC and VOC

The performance of components made of steel or cast iron that are free of coating with organic material is no emission of SVOC and VOC.

The performance of components of other materials has not been assessed.

Leachable substances

The product is not intended to be in direct contact to soil, ground water, and surface water.

3.3 Assessment methods

The assessment of the essential characteristics in Clause 3.1 of the BBR VT CONA CMM – Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands, for the intended uses, and in relation to the requirements for mechanical resistance and stability, safety in case of fire, and for hygiene, health and the environment, in the sense of the basic requirements for construction works № 1, 2, and 3 of Regulation (EU) № 305/2011, has been made in accordance with Annex A of EAD 160004-00-0301, Post-tensioning kits for prestressing of structures, for

- Item 1, Internal bonded tendon
- Item 2, Internal unbonded tendon

3.4 Identification

The European Technical Assessment for the BBR VT CONA CMM – Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands is issued on the basis of agreed data⁵ that identify the assessed product. Changes to materials, to composition or characteristics of the product, or to the production process could result in these deposited data being incorrect. Österreichisches Institut für Bautechnik should be notified before the changes are introduced, as an amendment of the European Technical Assessment is possibly necessary.

4 Assessment and verification of constancy of performance (hereinafter AVCP) system applied, with reference to its legal base

4.1 System of assessment and verification of constancy of performance

According to the Commission Decision 98/456/EC the system of assessment and verification of constancy of performance to be applied to the BBR VT CONA CMM – Unbonded and bonded

⁵ The technical file of the European Technical Assessment is deposited at Österreichisches Institut für Bautechnik.

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Post-tensioning System with 01, 02, and 04 Strands is System 1+. System 1+ is detailed in Commission Delegated Regulation (EU) № 568/2014 of 18 February 2014, Annex, point 1.1., and provides for the following items.

- (a) The manufacturer shall carry out
 - (i) factory production control;
 - (ii) further testing of samples taken at the manufacturing plant by the manufacturer in accordance with the prescribed test plan⁶.
- (b) The notified product certification body shall decide on the issuing, restriction, suspension or withdrawal of the certificate of constancy of performance of the construction product on the basis of the outcome of the following assessments and verifications carried out by that body
 - an assessment of the performance of the construction product carried out on the basis of testing (including sampling), calculation, tabulated values, or descriptive documentation of the product;
 - (ii) initial inspection of the manufacturing plant and of factory production control;
 - (iii) continuing surveillance, assessment, and evaluation of factory production control;
 - (iv) audit-testing of samples taken by the notified product certification body at the manufacturing plant or at the manufacturer's storage facilities.

4.2 AVCP for construction products for which a European Technical Assessment has been issued

Notified bodies undertaking tasks under System 1+ shall consider the European Technical Assessment issued for the construction product in question as the assessment of the performance of that product. Notified bodies shall therefore not undertake the tasks referred to in Clause 4.1, point (b) (i).

5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable EAD

5.1 Tasks for the manufacturer

5.1.1 Factory production control

The kit manufacturer exercises permanent internal control of the production. All the elements, procedures, and specifications adopted by the kit manufacturer are documented in a systematic manner in the form of written policies and procedures.

- Control of the incoming materials
 - The manufacturer checks the incoming materials to establish conformity with their specifications.
- Inspection and testing

Kind and frequency of inspections, tests, and checks, conducted during production and on the final product normally include.

- Definition of the number of samples taken by the kit manufacturer
- Material properties e.g. tensile strength, hardness, surface finish, chemical composition, etc.
- Determination of the dimensions of components

⁶ The prescribed test plan has been deposited with Österreichisches Institut für Bautechnik and is handed over only to the notified product certification body involved in the procedure for the assessment and verification of constancy of performance. The prescribed test plan is also referred to as control plan.

- Check correct assembly
- Documentation of tests and test results

All tests are performed according to written procedures with suitable calibrated measuring devices. All results of inspections, tests, and checks are recorded in a consistent and systematic way. The basic elements of the prescribed test plan are given in Annex 23, conform to EAD 160004-00-0301, Table 3, and are specified in the quality management plan of the BBR VT CONA CMM – Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands.

The results of inspections, tests, and checks are evaluated for conformity. Shortcomings request the manufacturer to immediately implements measures to eliminate the defects.

Control of non-conforming products

Products, which are considered as not conforming to the prescribed test plan, are immediately marked and separated from such products that conform. Factory production control addresses control of non-conforming products.

- Complaints

Factory production control includes procedures to keep records of all complaints about the PT system.

The records are presented to the notified product certification body involved in continuous surveillance and are kept at least for ten years after the product has been placed on the market. On request, the records are presented to Österreichisches Institut für Bautechnik.

At least once a year the manufacturer audits the manufacturers of the components given in Annex 24.

5.1.2 Declaration of performance

The manufacturer is responsible for preparing the declaration of performance. When all the criteria of the assessment and verification of constancy of performance are met, including the certificate of constancy of performance issued by the notified product certification body, the manufacturer draws up the declaration of performance. Essential characteristics to be included in the declaration of performance for the corresponding intended use are given in Table 9. In Annex 25 the combinations of essential characteristics and corresponding intended uses are listed.

5.2 Tasks for the notified product certification body

5.2.1 Initial inspection of the manufacturing plant and of factory production control

The notified product certification body establishes that, in accordance with the prescribed test plan, the manufacturing plant, in particular personnel and equipment, and the factory production control are suitable to ensure a continuous manufacturing of the PT system according to the given technical specifications. For the most important activities, EAD 160004-00-0301, Table 4 summarises the minimum procedure.

5.2.2 Continuing surveillance, assessment and evaluation of factory production control

The activities are conducted by the notified product certification body and include surveillance inspections. The kit manufacturer is inspected at least once a year. Factory production control is inspected and samples are taken for independent single tensile element tests.

For the most important activities, the control plan according to EAD 160004-00-0301, Table 4 summarises the minimum procedure. It is verified that the system of factory production control and the specified manufacturing process are maintained, taking account of the control plan.

Each manufacturer of the components given in Annex 24 is audited at least once in five years. It is verified that the system of factory production control and the specified manufacturing process are maintained, taking account of the prescribed test plan.

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The results of continuous surveillance are made available on demand by the notified product certification body to Österreichisches Institut für Bautechnik. When the provisions of the European Technical Assessment and the prescribed test plan are no longer fulfilled, the certificate of constancy of performance is withdrawn by the notified product certification body

5.2.3 Audit-testing of samples taken at the manufacturing plant or at the manufacturer's storage facilities

During surveillance inspection, the notified product certification body takes samples of components of the PT system for independent testing. Audit-testing is conducted at least once a year by the notified product certification body. For the most important components, Annex 24 summarises the minimum procedures. Annex 24 conforms to EAD 160004-00-0301, Table 4. In particular, at least once a year, the notified product certification body also carries out one single tensile element test series according to EAD 160004-00-0301, Annex C.7 and Clause 3.3.4 on specimens taken from the manufacturing plant or at the manufacturer's storage facility.

Issued in Vienna on 08 June 2018 by Österreichisches Institut für Bautechnik

The original document is signed by

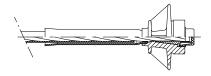
Rainer Mikulits Managing Director Page 30 of European Technical Assessment ETA-06/0165 of 08.06.2018, replaces European Technical Assessment ETA-06/0165 of 22.07.2016 and European technical approval ETA-12/0282 with validity from 26.06.2013 to 25.06.2018



Stressing anchorages, accessible fixed anchorages SA, FA

CONA CMM S1-0106 (single)

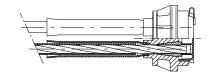






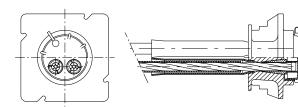
CONA CMM S1-0106 (two) 1)

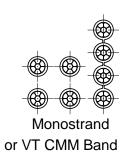






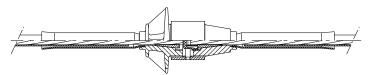
CONA CMM S1-0406 (four) 1)



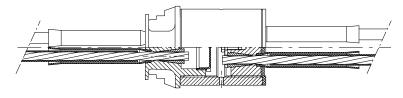


Fixed and stressing couplers FH, SH

Sleeve coupler H CONA CMM S1-0106 (single)



Sleeve coupler H CONA CMM S1-0406 (four)



VT CMM Band

VT CMM Band 01-140 VT CMM Band 01-150 VT CMM Band 02-140 VT CMM Band 02-150

VT CMM Band 04-140 VT CMM Band 04-150



VT CMM Band 01-165

Triangle VT CMM Band 02-165

VT CMM Band 04-165



1) Wedges secured with wedge retaining plates are also available.



Internal Post-tensioning System

Overview on anchorages and fixed couplers - S1

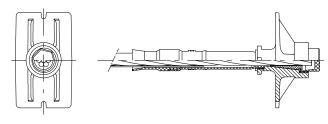
Annex 1

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Stressing anchorage, accessible fixed anchorage SA, FA

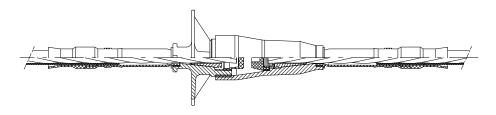
CONA CMM S2-0106 (single)





Fixed and stressing coupler FT, ST

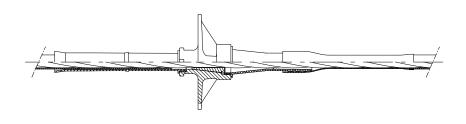
Coupler T CONA CMM S2-0106 (single)





Stressing intermediate anchorage SIA

SIA CONA CMM S2-0106 (single)





VT CMM Band

VT CMM Band 01-140 VT CMM Band 01-150





Internal Post-tensioning System

Overview on anchorages and fixed couplers – S2

Annex 2

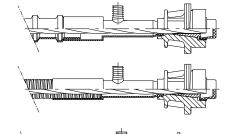
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Stressing anchorages, accessible fixed anchorages SA, FA

CONA CMM S1-0106 (single)

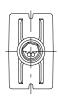


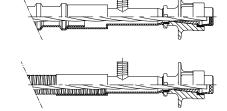




or VT CMM Band

CONA CMM S2-0106 (single)

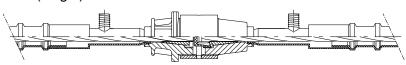




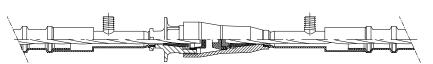


Fixed and stressing couplers FH, SH

H CONA CMM S1-0106 (single)

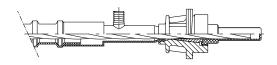


T CONA CMM S2-0106 (single)

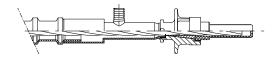


Restressable and exchangeable anchorages SA, FA

CONA CMM S1-0106 (single)









Internal Post-tensioning System

Overview on anchorages and couplers Unbonded and bonded tendon with duct – S1, S2

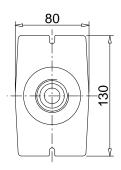
Annex 3

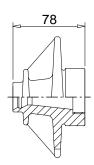
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Stressing and fixed anchorages

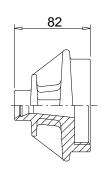
(S/F) A CONA CMM S1-0106 (single)



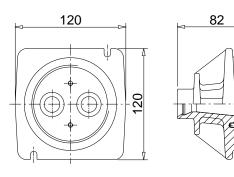


(S/F) A CONA CMM S1-0206 (two) A1

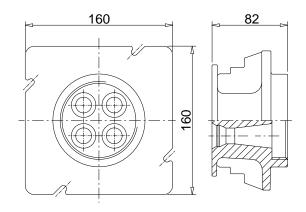
120



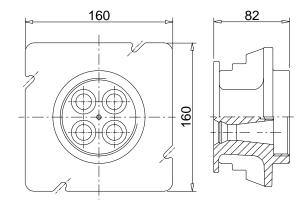
(S/F) A CONA CMM S1-0206 (two) A2



(S/F) A CONA CMM S1-0406 (four) A1



(S/F) A CONA CMM S1-0406 (four) A2



Dimensions in mm



Internal Post-tensioning System

Components - Stressing and fixed anchorage - S1

Annex 4

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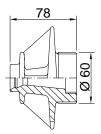


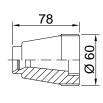
Fixed couplers

H CONA CMM S1-0106 (single)

1. BA

2. BA

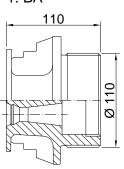


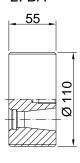


H CONA CMM S1-0406 (four)

1. BA

2. BA

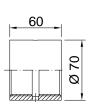


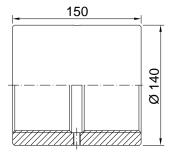


Threaded coupler sleeves

H CONA CMM S1-0106 (single)

H CONA CMM S1-0406 (four)

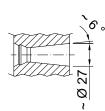


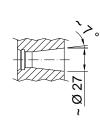


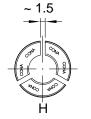
Cast iron cone S1

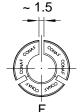
Machined cone

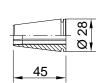
Ring wedges











Dimensions in mm



Internal Post-tensioning System

Components - Fixed coupler - S1

Annex 5

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Stressing and fixed anchorage

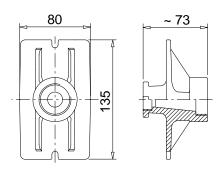
A CONA CMM S2-0106

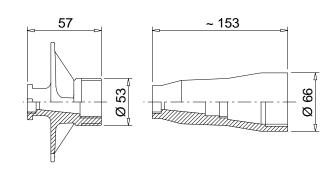
Fixed coupler

T CONA CMM S2-0106

1. BA

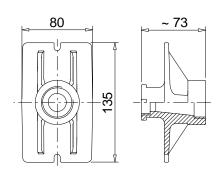
2. BA



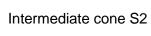


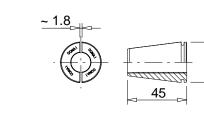
Stressing intermediate anchorage SIA

SIA CONA CMM S2-0106

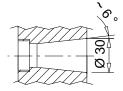


Cast iron cone S2





Ring wedge



Dimensions in mm



Internal Post-tensioning System

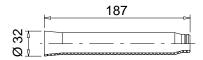
Components - Anchorages and fixed coupler - S2

Annex 6

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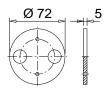
Transition pipe S1

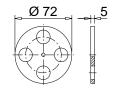


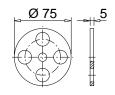
Wedge holding plate

A CONA CMM S1-0206 (two) A2 A CONA CMM S1-0406 (four) A2

H CONA CMM S1-0406 (four), 2. BA

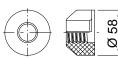




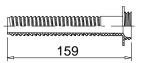


Pocket former set

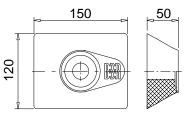
Nut S1-0106, S2-0106



Mandrel S1-0106, S2-0106

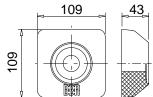


Pocket former R S1-0106, R S2-0106



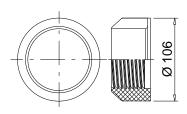
Nut S1-0206, S1-0406

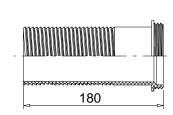
Pocket former C S1-0106

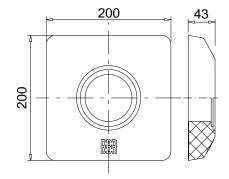


Mandrel \$1-0206 \$1-0406

Pocket former C S1-0206 C S1-0406







Dimensions in mm



Internal Post-tensioning System

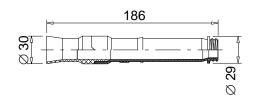
Components – Accessory

Annex 7

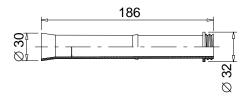
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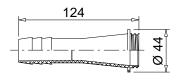
Transition pipe A-S2



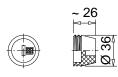
Transition pipe I-S2



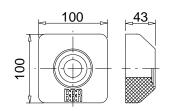
Protection pipe-S2



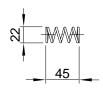
Coupler plug CONA CMM S2-0106



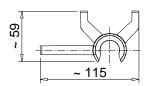
Pocket former C S2-0106



Spring A



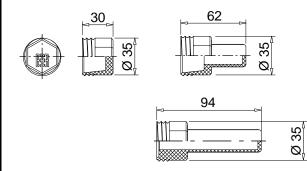
Reinforcement chair CONA CMM S2-0106





Protection caps

CONA CMM S1-0106 (single), CONA CMM S2-0106 (single) Protection caps with holes for ventilation are also available.



Dimensions in mm



Internal Post-tensioning System

Components - Accessory

Annex 8

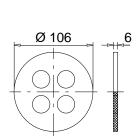
electronic copy

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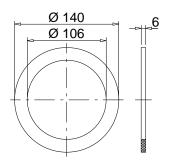


BDSD-plate

CONA CMM S1-406 (four) Version A





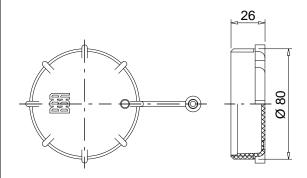




Wedge holding ring

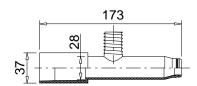
Protection caps

CONA CMM S1-0206 (two) CONA CMM S1-0406 (four)

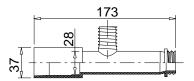


(Grouting) Transition pipe steel duct A

CONA CMM S1-0106 (single)

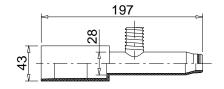


CONA CMM S2-0106 (single)

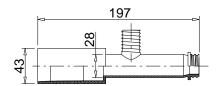


(Grouting) Transition pipe plastic duct A

CONA CMM S1-0106 (single)



CONA CMM S2-0106 (single)



Dimensions in mm



Internal Post-tensioning System

Components - Accessory

Annex 9





Material characteristics

Component	Standard / Specification
Anchor head S1-0106/0206/0406 Anchor head S2-0106	EN 1563
Coupler anchor head S1-0106/0406 – 1. BA Coupler anchor head S2-0106 – 1. BA	EN 1563
Coupler anchor head S1-0106 – 2. BA Coupler anchor head S2-0106 – 2. BA	EN 1563 EN 10083-1 EN 10083-2
Coupler anchor head S1-0406 – 2. BA	EN 10083-1 EN 10083-2
Coupler sleeve S1-0106/0406 Coupler sleeve S1-0106	EN 10210-1
Ring wedge H F I	EN 10277-2 EN 10084
Wedge holding plate	EN 10025-2
Helix	Ribbed reinforcing steel, R _e ≥ 500 MPa
Additional reinforcement, stirrups	Ribbed reinforcing steel, $R_e \ge 500 \text{ MPa}^{1)}$
Transition pipes S1 Transition pipes S2 A and I Protection pipes S1 Protection pipes S2	EN ISO 16396-1 EN ISO 17855-1
Grout pipe	EN ISO 16396-1 EN ISO 17855-1
Wedge holding ring Protection caps S1 and S2 Coupler plug CONA CMM S2-0106 Pocket former sets	EN ISO 17855-1
Spring A	EN 10270-1
Steel strip sheath	EN 523
Smooth steel duct	EN 10255, EN 10216-1, EN 10217-1, EN 10219-1, EN 10305-5
Smooth plastic duct	EN 12201-1
BDSD-plate	_

Ribbed reinforcing steel with $R_{\text{e}}\!\geq\!460\,\text{MPa}$ may be placed according to Annex 14 and Annex 15.



Internal Post-tensioning System Material specifications

Annex 10



7-wire prestressing steel strands according to prEN 10138-3 1)

									
Steel designation			Y1770S7	Y1860S7	Y1770S7	Y1860S7	Y1820S7G		
Tensile strength	R_{m}	MPa	1 770	1 860	1 770	1 860	1 820		
Diameter	D	mm	15.3	15.3	15.7	15.7	15.2 ²⁾		
Nominal cross-sectional area	A_p	mm²	140	140	150	150	165		
Nominal mass per metre	М	kg/m	1.0)93	1.1	172	1.289		
Permitted deviation from n mass	ominal	%	½ ± 2						
Characteristic value of maximum force	F_{pk}	kN	248	260	266	279	300		
Maximum value of maximum force	F _{m, max}	kN	285	299	306	321	345		
Characteristic value of 0.1 % proof force 3)	F _{p0.1}	kN	218	229	234	246	264		
$\begin{array}{c} \text{Minimum elongation at} \\ \text{maximum force,} \\ L_0 \geq 500 \text{ mm} \end{array}$	A_{gt}	%	3.5						
Modulus of elasticity	Ep	MPa	a 195 000 ⁴⁾						

- 1) Suitable prestressing steel strands according to standards and regulations in force at the place of use may also be used.
- Compacted strand
- For prestressing steel strands according to prEN 10138-3, 09.2000, the values are multiplied by 0.98.
- Standard value



Internal Post-tensioning System Specifications of prestressing steel strands

Annex 11



CONA CMM S1-0106/0206/0406-140 and CONA CMM S2-0106-140

Number of strands	Nominal cross- sectional area of	Nominal mass of prestressing steel	Nominal mass of	Characteristic va	alue of maximum tendon
oi stranus	prestressing steel	prestressing steer	tendon	f _{pk} = 1 770 MPa	f _{pk} = 1 860 MPa
n	Ap	М	М	F _{pk}	F_{pk}
_	mm²	kg/m	kg/m	kN	kN
01	140	1.09	1.23	248	260
02	280	2.19	2.46	496	520
04	560	4.37	4.92	992	1 040

CONA CMM S1-0106/0206/0406-150 and CONA CMM S2-0106-150

Number of strands	Nominal cross- sectional area of	Nominal mass of	Nominal mass of		alue of maximum tendon
UI SII AI IUS	prestressing steel	I Draetraeeina etaal		f _{pk} = 1 770 MPa	f _{pk} = 1 860 MPa
n	Ap	M	M	F _{pk}	F_{pk}
	mm²	kg/m	kg/m	kN	kN
01	150	1.17	1.31	266	279
02	300	2.34	2.62	532	558
04	600	4.69	5.24	1 064	1 116

CONA CMM S1-0106C/0206C/0406C-165, Compacted strand

Number of strands	Nominal cross- sectional area of prestressing steel	Nominal mass of prestressing steel	Nominal mass of tendon	Characteristic value of maximum force of tendon $f_{pk} = 1820 \text{ MPa}$
n	A_p	M	М	F _{pk}
	mm²	kg/m	kg/m	kN
01	165	1.29	1.42	300
02	300	2.58	2.84	600
04	660	5.16	5.68	1 200



Internal Post-tensioning System
Tendon ranges

Annex 12

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Maximum prestressing and overstressing forces

CONA CMM S1-0106 (C) (single)

Prestressing steel strands		A_P	mm ²	14	10	15	50	165 ¹⁾
Characteristic tensile strength		f_{pk}	MPa	1 770	1 860	1 770	1 860	1 820
Characteristic value of maximum force of tel	ndon	F_{pk}	kN	248	260	266	279	300
Maximum prestressing force 2)	0.90	· F _{p0.1}	kN	196	206	211	221	238
Maximum overstressing force 2), 3)	0.95	· F _{p0.1}	kN	207	218	222	234	251

CONA CMM S2-0106 (single)

Prestressing steel strands	A_P	mm²	14	10	150		
Characteristic tensile strength		f_{pk}	MPa	1 770	1 860	1 770	1 860
Characteristic value of maximum force of te	endon	F_{pk}	kN	248	260	266	279
Maximum prestressing force 2)	0.90	· F _{p0.1}	kN	196	206	211	221
Maximum overstressing force 2), 3)	0.95	· F _{p0.1}	kN	207	218	222	234

CONA CMM S1-0206 (C) (two)

Prestressing steel strands		A _P	mm ²	14	10	15	50	165 ¹⁾
Characteristic tensile strength		f_{pk}	MPa	1 770	1 860	1 770	1 860	1 820
Characteristic value of maximum force of t	endon	F_{pk}	kN	496	520	532	558	600
Maximum prestressing force 2)	0.90	· F _{p0.1}	kN	392	412	421	443	475
Maximum overstressing force 2), 3)	0.95	· F _{p0.1}	kN	414	435	445	467	502

CONA CMM S1-0406 (C) (four)

Prestressing steel strands		A _P	mm²	14	10	15	50	165 ¹⁾
Characteristic tensile strength		f_{pk}	MPa	1 770	1 860	1 770	1 860	1 820
Characteristic value of maximum force of t	endon	F_{pk}	kN	992	1 040	1 064	1 116	1 200
Maximum prestressing force 2)	0.90	· F _{p0.1}	kN	785	824	842	886	950
Maximum overstressing force 2), 3)	0.95	· F _{p0.1}	kN	828	870	889	935	1 003

- 1) Compacted strand
- The given value are maximum values according to Eurocode 2. The actual values are taken from the standards and regulations in force at the place of use. Conformity with the stabilisation and crack width criteria in the load transfer test has been verified to a level of 0.80 · F_{pk}.

Where F_{pk}Characteristic value of maximum force of tendon $F_{p0.1}$Characteristic value of 0.1 % proof force of tendon

For prestressing steel strands according to prEN 10138-3, 09.2000, the values are multiplied by 0.98.

Overstressing is permitted if the force in the prestressing jack can be measured to an accuracy of \pm 5 % of the final value of the prestressing force.



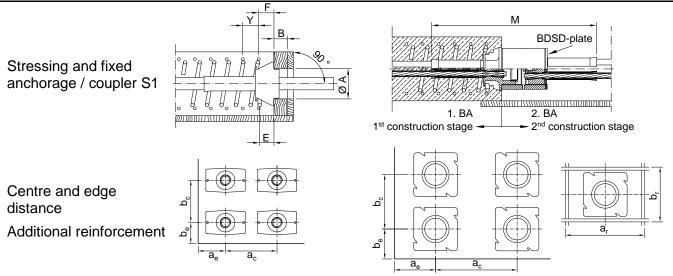
Internal Post-tensioning System

Maximum prestressing and overstressing forces

Annex 13

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Technical data of the BBR VT CONA CMM S1 anchorage and coupler anchorage system

Technical data of the BBR VT CONA CMM S1 anchorage and coupler anchorage system																
BBR VT CONA CMM 1)			S1-0	106 (siı	ngle)			S1-	0206 (t	wo)			S1-	0406 (f	our)	
	mm^2	14		15	50	165	14	10	15	50	165	14	40	15	50	165
	MPa	1770	1860	1770	1860	1820	1770	1860	1770	1 860	1820	1770	1860	1770	1860	1820
Char. value of F _{pk}	kN	248	260	266	279	300	496	520	532	558	600	992	1 040	1 064	1116	1 200
maximum force		_														
$0.90 \cdot F_{p0.1k}^{2)}$	kN	196	206	211	221	238	392	412	421	443	475	785	824	842	886	950
0.95 · F _{p0.1k} ²⁾	kN	207	218	222	234	251	251 414 435 445 467 502 828 870 889 935						935	1 003		
Dimensions of strands /	mm			Ø 20				2 × Ø	20 / 44	1 × 20			4 × Ø	20 / 90) × 20	
band							2 \ \(\infty \ 20 \/ \ 4 \ \(\infty \ 20 \/ \ 30 \ \(\infty \ 20 \/ \)									
Minimum concrete strer																
	MPa								≥ 24							
	MPa						1		≥ 20							
Helix, anchorage sided en	nd is								Ribb	ed reinf	orcina s	steel R	L _e ≥ 500	MPa		
welded	1									00 101111	oroning t	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	e = 000			
Outer diameter	mm								100					160		
Bar diameter	mm								10					12		
Length, approx. Pitch	mm								180			275 50				
	mm								40			50 5 + 1				
Number of pitches Distance E	mm								4 + 1 50			5 + 1 45				
	1					Б.		. , .		D . E	00.140					
Additional reinforcemen	t"			2		Ri	obed re	inforcin		, R _e ≥ 5	00 MPa	a *'		6		
Number of stirrups Bar diameter	mm			8					<u>4</u> 10					10		
Spacing Y	mm			50					50					55		
Distance from anchor																
plate	mm			55					25					53		
Outside dimensions a _r	mm			140					180					260		
b _r	mm			100					130					180		
Centre and edge spacing																
Minimum centre ac	mm			180					200					300		
spacing b _c	mm			140			150 220									
Minimum edge ae				70 + c			90 + c 130 + c						:			
distance b _e	mm			50 + c			65 + c 90 + c									
Bore in forms ØA	mm	65					103					103				
Bore in forms for coupler 1. BA	mm			62												
Depth B	mm			50					50					50		
Coupler M	mm			~ 545										~ 565		

- 1) For strand pattern see Annex 4.
- ²⁾ For prestressing steel strands according to prEN 10138-3, 2000, the values are multiplied by 0.98.
- 3) Additional reinforcement may be replaced by a rectangular helix of identical characteristics and number of pitches equal to number of stirrups plus one.
- Reinforcing steel with $R_e \ge 460$ MPa requires a stirrup spacing of 40 mm and one additional stirrup.

cConcrete cover



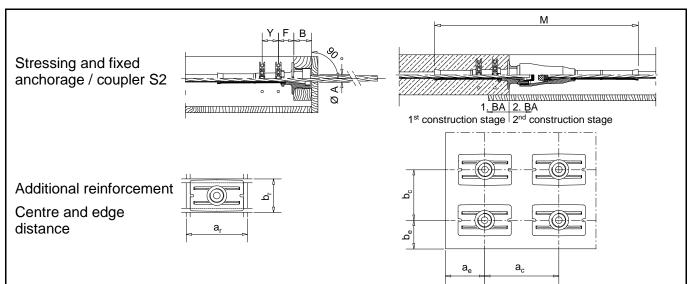
Internal Post-tensioning System

Dimensions of anchorages, helix, and additional reinforcement, centres spacing and edge distance Anchorages and couplers S1

Annex 14

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Technical data of the BB	Technical data of the BBR VT CONA CMM S2 anchorage, coupler anchorage and intermediate anchorage system													
BBR VT CONA CMM 1)				32-0106	(single	:)	S2-0106 (single)				S2-0106 (single)			
Strands	A_{P}	mm^2	140 150			140 150			140		15	50		
Char. tensile strength	f_{pk}	MPa	1770	1860	1770	1860	1770	1860	1770	1 860	1770	1 860	1770	1 860
Char. value of maximum force	F_{pk}	kN	248	260	265	279	248	260	265	279	248	260	265	279
$0.90 \cdot F_{p0.1k}^{2)}$		kN				192	206	206	216					
0.95 · F _{p0.1k} ²⁾		kN	202 218 218 228 202 218 218 228 202 218						218	228				
Dimensions of strands / band		mm		Ø	20			Ø	20			Ø	20	
Minimum concrete strength														
Cube	f _{cm, 0}	MPa				≥ :	22					≥	23	
Cylinder	f _{cm, 0}	MPa				≥	18					≥	19	
Helix 3), anchorage sided end is	weld	ed		Ribb	ed reinf	orcing s	teel, R _e	≥ 500 M	Pa ³⁾					
Outer diameter		mm		155	5/85									
Bar diameter		mm		1	0									
Length, approx.		mm		16	35									
Pitch		mm			5									
Number of pitches		_			3									
Distance	Е	mm		4	0									
Additional reinforcement 4)						Rib	bed rein	forcing s	steel, R _e	≥ 500 N	/IPa⁴)			
Number of stirrups		_							2					
Bar diameter		mm							0					
Spacing	<u>Y</u>	mm							5					
Distance from anchor plate	F	mm							0					
Outside dimensions	<u>a</u> r	mm							55					
	br	mm						8	5					
Centre and edge spacing														
Minimum centre spacing	<u>a</u> c	mm			75				75				35	
	b _c	mm)5)5				50	
Minimum edge distance	a _e	mm		78 + c					+ C				3 + C	
<u>-</u>	b _e	mm		43 + c			43 + c					+ C		
Bore in forms	\emptyset A				3		53						3	
Bore in forms for coupler 1. BA		mm		55			55			55				
Depth	В	mm			0				0				0	
Coupler	M	mm		~ 5	67			~ 5	67			~ {	567	

- For strand pattern see Annex 6.
- For prestressing steel strands according to prEN 10138-3, 2000, the values are multiplied by 0.98.
- Reinforcing steel with $R_e \ge 460$ MPa requires a pitch of 40 mm and one additional turn.
- Reinforcing steel with $R_e \ge 460$ MPa requires a stirrup spacing of 40 mm and one additional stirrup.
- c Concrete cover, Concrete cover refers to reinforcement, placed in the same cross section, however 10 mm as a minimum.



Internal Post-tensioning System

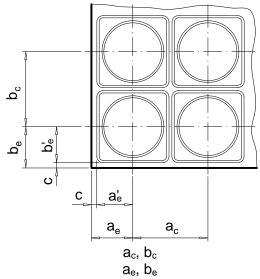
Dimensions of anchorages, helix, and additional reinforcement, centres spacing and edge distance Anchorages and coupler S2

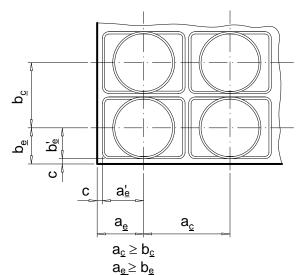
Annex 15

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Centre spacing and edge distance





Modification of centre spacing and edge distance are in accordance with the Clauses 1.8 and 2.2.3.4.

$$b_{\underline{c}} \geq \left\{ \begin{array}{l} 0.85 \cdot b_c \\ and \\ \geq \text{Helix, outside diameter} \end{array} \right.^{1)}$$

$$a_{\underline{c}} \geq \frac{A_c}{b_{\underline{c}}}$$

$$A_c = a_c \cdot b_c \le a_c \cdot b_c$$

Corresponding edge distances

$$a_{\underline{e}} = \frac{a_{\underline{c}}}{2} - 10 \text{ mm} + c$$

and

$$b_{\underline{e}} = \frac{b_{\underline{c}}}{2} - 10 \text{ mm} + c$$

c..... Concrete cover

The outer dimensions of the additional reinforcement are adjusted accordingly. Further modifications of reinforcement are in accordance with the Clause 1.12.5 and 2.2.3.4.



Internal Post-tensioning System

Anchorage zone – Dimensions Modification of centre spacing and edge distance Annex 16

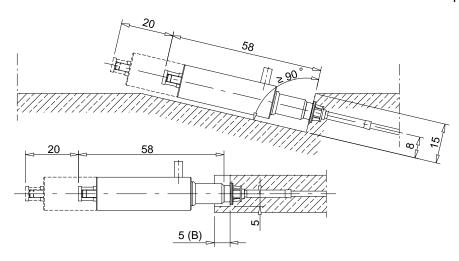


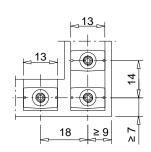
Construction examples, minimised anchorage recess dimensions

Tendon CONA CMM S1-0106 (single)

Strand excess length: 70 cm

Supposed 2 cm concrete cover

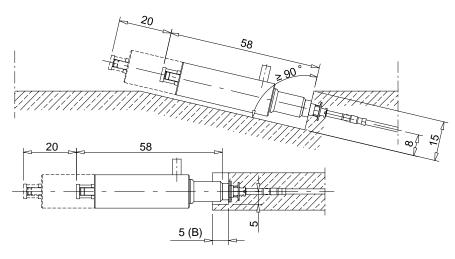


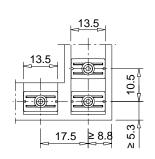


Tendon CONA CMM S2-0106 (single)

Strand excess length: 70 cm

Supposed 1 cm concrete cover





If other prestressing jacks will be used, different minimum dimensions may apply. Ask ETA holder for advice.

Dimensions in cm



Internal Post-tensioning System

Dimensions of anchorage recesses – S1, S2

Annex 17

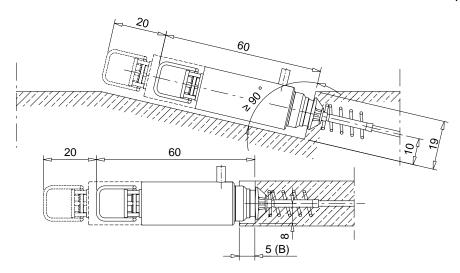
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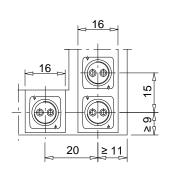


Construction examples, minimised anchorage recess dimensions

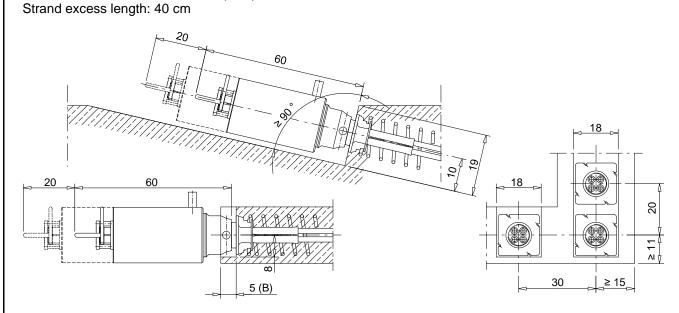
Tendon CONA CMM S1-0206 (two) Strand excess length: 40 cm

Supposed 2 cm concrete cover





Tendon CONA CMM S1-0406 (four)



If other prestressing jacks will be used, different minimum dimensions may apply. Ask ETA holder for advice.

Dimensions in cm



Internal Post-tensioning System

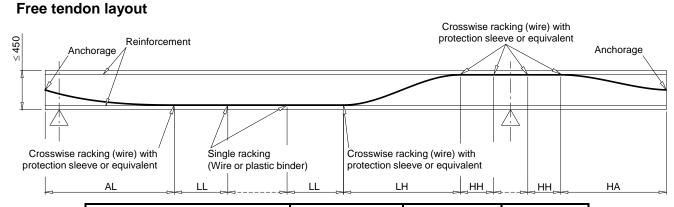
Dimensions of anchorage recesses – S1

Annex 18

electronic copy

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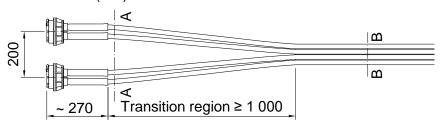




	Typical zones	Max. distances	Min. number	Racking
AL	Anchor – Low point	3 m	_	crosswise
LL	Low point – Low point	(1.0–1.3) m	2	single wire
LH	Low point – High point	3 m		crosswise
НН	High point – High point	(0.3–1.0) m	2	crosswise
НА	High point- Anchor	1.5 m		crosswise

Transition regions

Tendon CONA CMM S1-0206 (two)

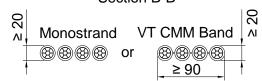




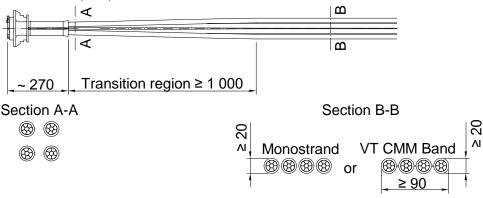
Section B-B







Tendon CONA CMM S1-0406 (four)



Dimensions in mm



Internal Post-tensioning System

Free tendon layout - Transition regions

of European Technical Assessment ETA-06/0165 of 08.06.2018

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PT Works A) Manufacturing of the tendon Construction Works Longitudinal cutting of the VT CMM Band 1) Removing the PE-sheathing at the end 2) 1) Wrapping the single prestressing steel strands with bandage filled with corrosion protective filling material in the region of the transition pipe Mounting the anchor block on the prestressing steel strands 2) 3) Prelocking the anchor block 4) Securing wedges with wedge holding rings or holding plate 2) 5) Filling protection cap with corrosion protective filling material and screwing it onto the anchor block Coiling according to the tendon list and fixing the tendon for transport/ Transport 3) B) Preliminary site works Erection of the formwork Fixing the active anchor block (SA, H 1. BA) on the formwork Placing reinforcement bottom layer and supporting stirrups C) Tendon installation Placing the tendon Fastening the tendon with wire or plastic binder at the bottom layer and supporting stirrups Coupling 4): The coupler anchor block 2. BA is mounted and prelocked on the prefabricated tendon Placing the coupler anchor block (2nd construction stage) Screwing-on the coupler sleeve on the coupler anchor 1. BA, meanwhile or afterwards filling of the space inside the coupler sleeve and both coupler anchor blocks with corrosion protective filling material. Connecting tendon with the active anchor block (SA, H 1.BA) Longitudinal cutting of the VT CMM Band 1) Removing the PE-sheathing at the end Wrapping the single prestressing steel strands with bandage filled with corrosion protective filling material in the region of the transition pipe Inserting the prestressing steel strands into the anchor block Putting on removed PE-sheathing to protect excess strand length 12) Placing reinforcement top layer 13) Fastening tendon with wire or plastic binder on the reinforcement top layer D) Concreting of the structure Concreting the structure, recommended to make testing cubes 2) Determining concrete strength Dismantling the formwork at the active anchorage side E) Stressing and finishing work Removal of protecting PE-sheaths and check whether cones are clean 2) Inserting ring wedges 3) Stressing the tendon according to stressing order 4) Cutting excess strand lengths Filling protection cap with corrosion protective filling material and screwing it onto the anchor block Filling of the anchorage recess with concrete 5) not applicable when assembling a tendon VT CMM 106 or not applicable in case of manufacturing on the site 4) only applicable when using a coupler applicable case of using an anchor body K VT CMM 406 – 2. BA ⁵⁾ not applicable when assembling a coupler 1st construction stage



Internal Post-tensioning System

Description of worksteps – Anchorage Fixed coupler 1st and 2nd construction stage

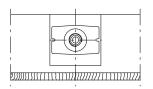
Annex 20

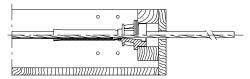
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Construction stages

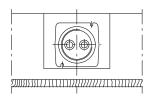
Tendon CONA CMM S1-0106 (single)

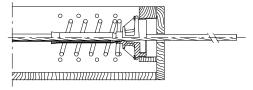


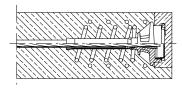




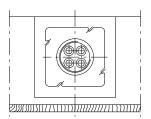
Tendon CONA CMM S1-0206 (two)

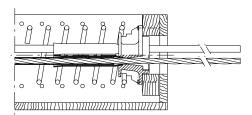


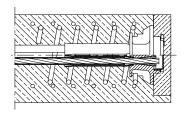




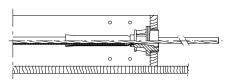
Tendon CONA CMM S1-0406 (four)

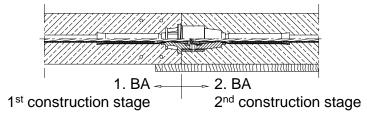




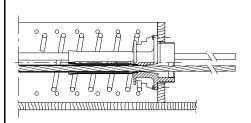


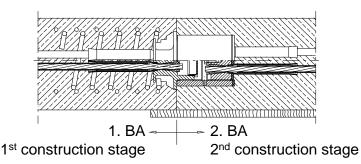
Fixed coupler CONA CMM S1-0106 (single)





Fixed coupler CONA CMM S1-0406 (four)







Internal Post-tensioning System

Construction stages
Anchorages and fixed couplers – S1

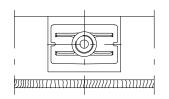
Annex 21

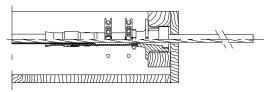
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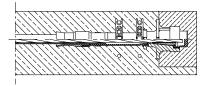


Construction stages

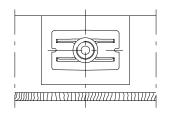
Tendon CONA CMM S2-0106 (with reinforcement)

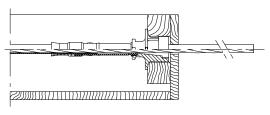


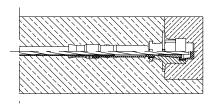




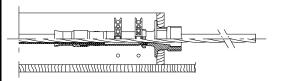
Tendon CONA CMM S2-0106 (without reinforcement)

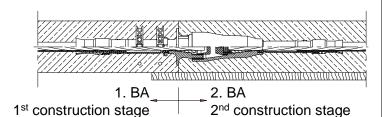




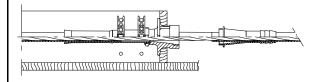


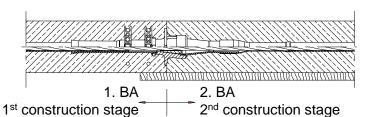
Coupler CONA CMM S2-0106





Intermediate CONA CMM S2-0106





BBR CONA CMM

Internal Post-tensioning System

Construction stages Anchorages and fixed couplers – S2 Annex 22



Contents of the prescribed test plan

Subject / type of control		Test of control method	Criteria, if any	Minimum number of samples	Minimum frequency of control
	Material	Checking 1)	2)	100 %	continuous
Anchor head, Coupler anchor head, Coupler sleeve	Detailed dimensions	Testing	2)	5 %, ≥ 2 specimens	continuous
	Visual inspection 3)	Checking	2)	100 %	continuous
	Traceability	full			
Ring wedge	Material	Checking 1)	2)	100 %	continuous
	Treatment, hardness	Testing	2)	0.5 %, ≥ 2 specimens	continuous
	Detailed dimensions	Testing	2)	5 %, ≥ 2 specimens	continuous
	Visual inspection 3)	Checking	2)	100 %	continuous
	Traceability	full			
VT CMM Band, Individual monostrand	Material	Checking	2), 4)	100 %	continuous
	Diameter	Testing	2), 4)	1 sample	each coil or
	Visual inspection	Checking	2), 4)	1 sample	every 7 tons 5)

- 1) Checking by means of an inspection report 3.1 according to EN 10204.
- ²⁾ Conformity with the specifications of the component
- 3) Successful visual inspection does not need to be documented.
- 4) Checking of relevant certificate as long as the basis of "CE"-marking is not available.
- 5) Maximum between a coil and 7 tons is taken into account.

Traceability full Full traceability of each component to its raw material.

Material Defined according to technical specification deposited by the supplier

Detailed dimension Measuring of all the dimensions and angles according to the specification given in the

test plan

Visual inspection Main dimensions, correct marking and labelling, surface, corrosion, coating, etc.

Treatment, hardness Surface hardness, core hardness, and treatment depth



Internal Post-tensioning SystemContents of the prescribed test plan

Annex 23



Audit testing

Subject / type of control		Test of control method	Criteria, if any	Minimum number of samples 1)	Minimum frequency of control
Anchor head, Coupler anchor head, Coupler sleeve	Material	Checking and testing, hardness and chemical ²⁾	3)	1	1/year
	Detailed dimensions	Testing	3)	1	1/year
	Visual inspection	Checking	3)	1	1/year
Ring wedge	Material	Checking and testing, hardness and chemical ²⁾	3)	2	1/year
	Treatment, hardness	Checking and testing, hardness profile	3)	2	1/year
	Detailed dimensions	Testing	3)	1	1/year
	Main dimensions, surface hardness	Testing	3)	5	1/year
	Visual inspection	Checking	3)	5	1/year
Single tensile element test		According to EAD 160004-00-0301, Annex C.7		9	1/year

- 1) If the kits comprise different kinds of anchor heads e.g. with different materials, different shape, different wedges, etc., then the number of samples are understood as per kind.
- ²⁾ Testing of hardness and checking of chemical composition by means of an inspection report 3.1 according to EN 10204.
- 3) Conformity with the specifications of the component

Material Defined according to technical specification deposited by the ETA holder at the

Notified body

Detailed dimension Measuring of all the dimensions and angles according to the specification given in

the test plan

Visual inspection Main dimensions, correct marking and labelling, surface, corrosion, coating, etc.

Treatment, hardness Surface hardness, core hardness and treatment depth



Internal Post-tensioning System
Audit testing

Annex 24



Nº	Essential Characteristic	Clause	Intended use Line № according to Clause 2.1, Table 8	
			1	2
1	Resistance to static load	3.2.1.1	+	+
2	Resistance to fatigue	3.2.1.2	+	+
3	Load transfer to the structure	3.2.1.3	+	+
4	Friction coefficient	3.2.1.4	+	+
5	Deviation, deflection (limits) for internal bonded and unbonded tendon	3.2.1.5	+	+
6	Assessment of assembly	3.2.1.6	+	+
7	Corrosion protection	3.2.1.7	+	+
8	Reaction to fire	3.2.2.1	+	+
9	Content, emission and/or release of dangerous substances	3.2.3.1	+	+

Key

+..... Essential characteristic relevant for the intended use

---.... Essential characteristic not relevant for the intended use

For combinations of intended uses, the essential characteristics of all intended uses composing the combination are relevant.



Internal Post-tensioning System Essential characteristics for the intended uses

Annex 25

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Reference documents

European Assessment Documents

EAD 160004-00-0301 Post-Tensioning Kits for Prestressing of Structures
EAD 160027-00-0301 Special filling products for post-tensioning kits

Standards	
Eurocode 2	Eurocode 2– Design of concrete structures
Eurocode 3	Eurocode 3 – Design of steel structures
Eurocode 4	Eurocode 4 – Design of composite steel and concrete structures
Eurocode 6	Eurocode 6 – Design of masonry structures
EN 206, 12.2013 + A1, 11.2016	Concrete: Specification, performance, production and conformity
EN 445, 10.2007	Grout for prestressing tendons – Test methods
EN 446, 10.2007	Grout for prestressing tendons – Grouting procedures
EN 447, 10.2007	Grout for prestressing tendons – Basic requirements
EN 523, 08.2003	Steel strip sheaths for prestressing tendons – Terminology, requirements, quality control
EN 1563, 12.2011	Founding – Spheroidal graphite cast irons
EN 10025-2, 11.2004 EN 10025-2/AC, 06.2005	Hot rolled products of structural steels – Part 2: Technical delivery conditions for non-alloy structural steels
EN 10083-1 08 2006	Steels for guenching and tempering — Part 1: General technical

EN 10083-1, 08.2006 Steels for quenching and tempering – Part 1: General technical delivery conditions

EN 10083-2, 08.2006 Steels for quenching and tempering - Part 2: Technical delivery

conditions for non alloy steels

EN 10084, 04.2008 Case hardening steels – Technical delivery conditions EN 10204, 10.2004 Metallic products – Types of inspection documents

EN 10210-1, 04.2006 Hot finished structural hollow sections of non-alloy and fine grain

steels - Part 1: Technical delivery conditions

EN 10216-1, 12.2013 Seamless steel tubes for pressure purposes – Technical delivery

conditions - Part 1: Non-alloy steel tubes with specified room

temperature properties

EN 10217-1, 05.2002 + Welded steel tubes for pressure purposes – Technical delivery conditions – Part 1: Non-alloy steel tubes with specified room

temperature properties

EN 10219-1, 04.2006 Cold formed welded structural hollow sections of non-alloy and fine

grain steels – Part 1: Technical delivery conditions

EN 10255:2004 + A1, Non-Alloy steel tubes suitable for welding and threading – Technical

04.2007 delivery conditions

EN 10270-1:2011 + A1, Steel wire for mechanical springs - Part 1: Patented cold drawn

05.2017 unalloyed spring steel wire



Internal Post-tensioning System

Reference documents

Annex 26

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Reference documents

Standards

EN 10277-2, 03.2008 Bright steel products – Technical delivery conditions – Part 2: Steels

for general engineering purposes

EN 10305-5, 03.2016 Steel tubes for precision applications – Technical delivery conditions

Part 5: Welded cold sized square and rectangular tubes

EN 12201-1, 09.2011 Plastics piping systems for water supply, and for drainage and

sewerage under pressure - Polyethylene (PE) - Part 1: General

EN ISO 16396-1, 02.2015 Plastics – Polyamide (PA) moulding and extrusion materials – Part 1:

Designation system, marking of products and basis for specifications

EN ISO 17855-1, 10.2014 Plastics - Polyethylene (PE) moulding and extrusion materials -

Part 1: Designation system and basis for specifications

prEN 10138-3, 09.2000 Prestressing steels – Part 3: Strand prEN 10138-3, 08.2009 Prestressing steels – Part 3: Strand

CWA 14646, 01.2003 Requirements for the installation of post-tensioning kits for

prestressing of structures and qualification of the specialist company

and its personnel

Other documents

98/456/EC Commission decision 98/456/EC of 3 July 1998 on the procedure for

attesting the conformity of construction products pursuant to Article 20 (2) of Council Directive 89/106/EEC as regards posttensioning kits for the prestressing of structures, Official Journal

of the European Communities L 201 of 17 July 1998, p. 112

305/2011 Regulation (EU) № 305/2011 of the European Parliament and of the

Council of 9 March 2011 laying down harmonised conditions for the marketing of construction products and repealing Council Directive 89/106/EEC, OJ L 88 of 4 April 2011, p. 5, amended by Commission Delegated Regulation (EU) № 568/2014 of 18 February 2014, OJ L 157 of 27.05.2014, p. 76 and Commission Delegated Regulation (EU) № 574/2014 of 21 February 2014, OJ L 159 of 28.05.2014, p. 41

568/2014 Commission Delegated Regulation (EU) № 568/2014 of 18 February

2014 amending Annex V to Regulation (EU) № 305/2011 of the European Parliament and of the Council as regards the assessment and verification of constancy of performance of construction products,

OJ L 157 of 27.05.2014, p. 76



Internal Post-tensioning System

Reference documents

Annex 27



Materialprüfungsamt Nordrhein-Westfalen

Prüfen · Überwachen · Zertifizieren

Certificate of constancy of performance 0432-CPR-00299-1.2 (EN)

Version 04

In compliance with Regulation (EU) No 305/2011 of the European Parliament and of the Council of 9 March 2011 (the Construction products Regulation or CPR), this certificate applies to the construction product

BBR VT CONA CMM - Unbonded and bonded Post-tensioning System with 01, 02, and 04 Strands

Bonded and unbonded post-tensioning kits for prestressing of structures with strands and monostrands

placed on the market under the name or trade mark of

BBR-VT International Ltd

Ringstr. 2 8603 Schwerzenbach (ZH) / Switzerland

and produced in the manufacturing plant(s)

BBR-VT International Ltd

Ringstr. 2 8603 Schwerzenbach (ZH) / Switzerland

This certificate attests that all provisions concerning the assessment and verification of constancy of performance described in the

ETA 06/0165, issued on 08.06.2018

and

EAD 160004-00-0301

under system 1+ for the performance set out in the ETA are applied and that the factory production control conducted by the manufacturer is assessed to ensure the

constancy of performance of the construction product.

This certificate was first issued on 07.09.2016 and will remain valid until 17.08.2028 as long as neither the ETA, the EAD, the construction product, the AVCP methods nor the manufacturing conditions in the plant are modified significantly, unless suspended or withdrawn by the notified product certification body.

Dortmund, 18.08.2023

by order

Ripl-Ing. I

Head of Certification Body (Dep. 21)

This Certificate consists of 1 page.

This Certificate replaces the Certificate no. 0432-CPR-00299-1.2 (EN) dated 23.07.2018, Version 03.

The original of this document was issued in German language.

In case of doubt only the German version is valid.

DAkkS

Deutsche
Akkreditierungsstelle
D-ZE-11142-01-00

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Technical Headquarters and Business Development Centre Switzerland

